









Corridor Traffic Review Report

East River Road - Erie Station Road to Jefferson Road

Town of Henrietta, Monroe County NY

April 2018







CORRIDOR TRAFFIC REVIEW REPORT

TABLE OF C	CONTENTS	
INTRODUCT	FION	1
PROJECT L	OCATION	2
EXISTING C	ONDITIONS	4
Existing Co	orridor	4
Existing Int	tersections	4
Pedestrian	/Bicycle Accommodations	5
Existing Tra	affic Voumes and Patterns	8
Existing Ca	apacity	10
FUTURE TR	ANSPORTATION DEMAND	10
Anticipated	Development	10
Area Grow	th	10
Committed	I Improvements	10
Future Traf	ffic Projections and Trip Distribution	14
Full Develo	ppment Volumes	16
CAPACITY A	ANALYSIS	25
Intersection	n Capacity Analysis	25
Intersection	n Capacity Improvements	32
Linear Corr	ridor Analysis	33
CONCLUSIO	ONS & RECOMMENDATIONS	34
Corridor Im	nprovements	34
Pedestrian	Accommodations	34
Bicycle Acc	commodations	35
Transit Ser	rvice	35
Other Cons	siderations	36
COST SHAR	RING	36
<u>APPENDICE</u>	<u>ss</u>	
Appendix A	Existing Traffic Counts	
Appendix B	Synchro Output	
Appendix C	Proposed Improvement Concepts	
Appendix D	Opinion of Probable Costs	

INTRODUCTION

As times have changed, so has East River Road. In years past, this winding 2-lane road (County Road 84), making its way through the fields and woods in the Town of Henrietta, was very rural in character. But with anticipated economic development the character of the East River Road corridor is beginning to transform.

As one moves through the East River Road corridor from north to south, you see clear evidence of this growth. At the north end for example, there is the Rochester Institute of Technology (RIT), which has become a nationally



renowned college that has seen great expansion, and thus is a driving force of the Greater Rochester economy. Recent additions include the Gordon Field House, and the Golisano Institute of Sustainability building, which is visible from East River Road. Continuing in a southerly direction, land uses along and adjacent to East River Road are primarily residential, with a few exceptions (e.g. RIT Business & Technology Park on Lucius Gordon Drive, vacant Kodak Riverwood campus near the intersection of Lehigh Station Road and East River Road, and minor commercial establishments).

East River Road is a key north south link in the overall transportation network servicing the area. This facility is experiencing increased vehicle traffic, along with bicycle and pedestrian traffic, from the development that is taking place immediately within the corridor, as well as beyond the study area.

The Town of Henrietta, in conjunction with the Monroe County Department of Transportation and RIT, commenced completion of a corridor traffic analysis in order to define current and future traffic conditions. The purpose of this study is to define potential traffic impacts and identify any operational deficiencies over the next 20 years to assist the Town and County in planning for future roadway improvements as growth and surrounding development occur.

This study looked at the section of East River Road from Erie Station Road to Jefferson Road which is 3.8 miles in length and includes six (6) major intersections. The analysis focused on the operations at these intersections and any impacts to linear capacity for the roadway segments in between. See Figures 1 and 2 for County and Town location maps.

The Town provided future development scenarios to predict future traffic volumes which combined with existing traffic volumes were used to investigate the current configuration and performance of the corridor as well as the future anticipated conditions. The East River Road Traffic Review project has three primary goals:

- Develop future traffic volumes for years 2020, 2025, 2030 and 2035, which will be established using existing volumes, background growth and anticipated corridor development,
- 2) Creating infrastructure conceptual alternatives to accommodate future growth, and
- 3) Providing a framework for identifying potential equitable infrastructure funding shares, based on traffic volumes, for the County, Town and private developers.

The following sections outline the steps taken to achieve the project goals.

PROJECT LOCATION



Figure 1 - County Map

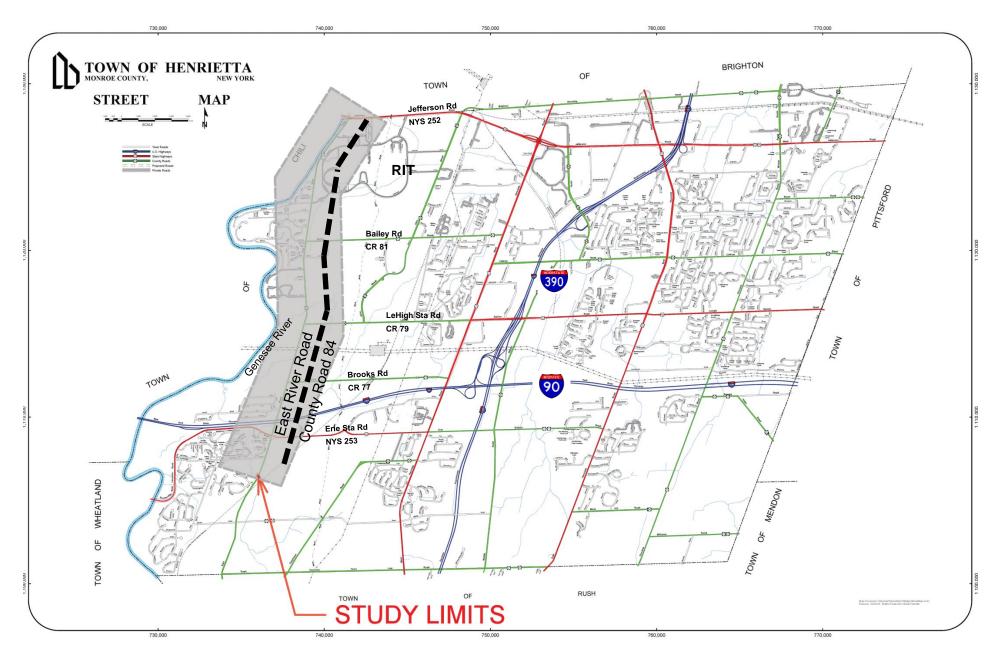


Figure 2 - Town Local Map

1 - EXISTING CONDITIONS

The East River Road (County Road 84) corridor is located in the Town of Henrietta, New York with the study area extending from Erie Station Road to the South to Jefferson Road to the North. A total of six (6) intersections are located within the project corridor:

- 1. Erie Station Road (NYS 253) at East River Road (CR 84) NYSDOT Jurisdiction
- 2. Brooks Road (CR 77) at East River Road (CR 84) MCDOT
- 3. Lehigh Station Road (CR 79) at East River Road (CR 84) MCDOT
- 4. Bailey Road (CR 81)/Chesapeake Landing at East River Road (CR 84) MCDOT
- 5. River Meadow/Farnum Lane (RIT) at East River Road (CR 84) MCDOT
- 6. Jefferson Road (NYS 252) at East River Road (CR 84) NYSDOT

1.1 Existing Corridor

The East River Road corridor is a north/south urban minor arterial roadway under the jurisdiction of the Monroe County Department of Transportation. The corridor is primarily a 2-lane roadway with turn lanes at some of the intersections and some short segments with multiple lanes. Excluding auxiliary lanes at the intersections the following segments contain more than two (2) travel lanes:

- Two (2) southbound lanes south of Jefferson Road for a distance of approximately 600'.
- Two-Way Center Turn Lane (TWCTL) between Still Pond Way/Lucius Gordon Drive and Chesapeake Landing/Bailey Road for a distance of approximately 900'.

The terrain within the corridor includes rolling hills and becomes more rural south of the Chesapeake Landing/Bailey Road intersection. The northern segment contains a large number of residential and commercial driveways north of Bailey Road. East River Road is adjacent to the Genesee River at the northern end of the study area. The posted speed limit within the study corridor is 40 miles per hour.

1.2 Existing Intersections

The following intersections (from South to North) are located within the study limits and will be part of the capacity analysis:

<u>East River Road (CR 84) at Erie Station Road (NYS 253)</u> – Is a four-way signalized intersection under the jurisdiction of NYSDOT located just south of the bridge over the Thruway. The Northbound East River Road approach is a single lane with a widened right shoulder. The southbound approach has two lanes beginning 200' from the intersection which merge into a single lane just south of the intersection. The Erie Station Road approaches include two lanes in each direction which merge to one lane beyond the intersection in both directions.

<u>East River Road at Brooks Road (CR 77)</u> – Is a three-way stop sign controlled intersection with a stop sign on the Brooks Road approach which is under the jurisdiction of MCDOT. The East River Road and Brooks Road approaches are 2-lanes.

<u>East River Road at Lehigh Station Road (CR 79)</u> – Is a three-way signalized intersection located just south of the former Kodak Riverwood facility main access which is under the jurisdiction of MCDOT. The southbound approach includes a dedicated left turn lane onto Lehigh Station road. The Lehigh Station Road approach provides a dedicated left and right turn lane. This intersection is offset 250' from the former Kodak Riverwood facility northern driveway.

<u>East River Road at Bailey Road/Chesapeake Landing</u> – Is a four-way signalized intersection under the jurisdiction of MCDOT. Northbound and southbound East River Road approaches have dedicated left turn lanes. Bailey Road (westbound) has a dedicated right turn lane and the EB Chesapeake Landing approach is a single lane.

East River Road at Farnum Lane/River Meadow Drive – Is a four-way signalized intersection. Both northbound and southbound approaches of East River Road are a single lane. The eastbound River Meadow drive approach is a single lane and the Farnum Lane (RIT) westbound approach has an additional dedicated right turn lane onto East River Road.

East River Road at Jefferson Road - Is a large four-way signalized intersection which is bordered by the Genesee River to the West. Both northbound and southbound East River Road approaches have dedicated left and right turn lanes onto Jefferson road and one through lane. The eastbound Jefferson Road approach has two through lanes and dedicated left and right turn lanes. The westbound Jefferson Road approach has a dedicated left turn lane and three through lanes, the right most lane becoming dedicated right turn lane onto Scottsville Road.

Pedestrian Accommodations

There are no dedicated sidewalks along East River Road within the study area. Pedestrians are currently using the available shoulders. The following residential side streets have sidewalk connections to East River Road:

- 1. Farrell Road
- 2. Chesapeake Landing
- 3. Cape Henry Trail
- 4. Residential Trail system at the Southwest corner of Erie Station Road

A short Trail segment is also located between East River Road and the Genesee River just south of Jefferson Road. Except for Jefferson Road there are no pedestrian crossings or pedestrian signals at the existing intersections. Pedestrian activity was seen in the corridor primarily north of Bailey Road.

Bicycle Accommodations

Bicyclists use the existing shoulders when traveling within the corridor. Existing shoulder widths vary from 1' to 10' with much of the corridor having 5-6' shoulders.





Pedestrian and Bicycle Users on East River Road

Table 1 – S	Summary of M	lainline Roadway	/ Segme	ents
Road Segment	Number of thru Lanes	Lane Type	Lane Width	Shoulder Width
South of Erie Station Rd.	2	Travel Lane	11'	3'
Erie Station Rd. to Brooks Rd.	2	Travel Lane	11'	NB varies 1' to 10' SB varies 1' to 3'
Brooks Rd. to Lehigh Station Rd.	2	Travel Lane	11'	6'
Lehigh Station Rd. to Bailey Rd./Chesapeake Landing	¹ 2	Travel Lane, Center turn lane	11'	NB varies 5' to 10' SB varies 3' to 8'
Bailey Rd./ Chesapeake Landing to River Meadow/Farnum Ln.	2	Travel Lane	11'	5'
Meadow/Farnum Ln. to Jefferson Rd.	2	Travel Lane	11'	Varies 5' to 6'
North of Jefferson Rd.	2	Travel Lane	11'	6'

¹A Center Two-Way Left Turn Lane is located between Still Pond Way/Lucius Gordon and Chesapeake/Bailey Road.

		Table 2 - East River Road Intersection Characteristics	ersection Cha	racteristics			0; ==1
Intersecting Road Location	No. Lanes	[Fa	Lane width	Turn Lane length	Shoulder width	Edge Treatment	Control Feature
		Erie Station Road	Road				
East River Road - North	3	2 SB travel lane, 1 NB travel lane	11,	315'	1'	none	
East River Road - South	2	1 travel lane each direction	II.		11' NB, 6' SB	none	1000
Erie Station Road - East	3	2 WB travel lanes, 1 EB travel lane	,11,	100,	3'	none	- ORIG
Erie Station Road - West	4	2 travel lanes each direction	11,	350'	1'	gutter	
		Brooks Road	oad				
East River Road - North	2	1 travel lane each direction	11,	=	3'	none	
East River Road - South	2	1 travel lane each direction	11'	1	1'	none	Stop Sign
Brooks Road - East	2	1 travel lane each direction	11,		1'	none	
		Lehigh Station Road	on Road				
East River Road - North	ю	1 SB dedicated left turn lane, 1 travel lane each direction	11,	100'	6' NB, 3' SB	none	Ü
East River Road - South	2	1 travel lane each direction	11'		.9	none	olgilal Serial
Lehigh Station Road	ĸ	Dedicated left and right turn lane, 1 EB travel lane	11,	125'	2' WB, 3' EB	none	
		Bailey Road/Chesapeake Landing	peake Landin	00			
East River Road - North	м	1 SB dedicated left turn lane, 1 travel lane each direction	11,	150'	SB varies from 2' to 5', NB 3'	none	
East River Road - South	3	1 NB dedicated left turn lane, 1 travel lane each direction	,11,	100'	4'	none	Signal
Bailey Road	3	1 WB dedicated right turn lane, 1 travel lane each direction	11'	200'	8' EB, 3' WB	none	
Chesepeake Landing	2	1 travel lane each direction	10,	-	No Shoulder	gutter	
		River Meadow Drive/Farnum Lane	e/Farnum Lar	e.			
East River Road - North	2	1 travel lane each direction	11,	- T-	5'	none	
East River Road - South	2	1 travel lane each direction	11'	(1)	9' NB, 5' SB	none	
Farnum Lane	е	 WB dedicated right turn lane, 1 travel lane each direction 	14' EB, 11' WB	100'	2' EB, 1' WB	none	Signal
River Meadow Drive	2	1 travel lane each direction	10,	-	No Shoulder	none	
	i i	Jefferson Road	Road				
East River Road - North	4	Dedicated left and right SB turn lanes, 1 Sb thru lane, 1 NB travel lane	11,	275'	5'	none	
East River Road - South	5	Dedicated NB right and left turn lanes, 2 SB travel lanes, 1 NB thru lane	11,	300' right turn, 800' left turn	6'	none	- G
Jefferson Road - East	9	Dedicated WB left turn lane, 3 WB and 2 EB travel lanes	12'	350'	3'	curb	
Jefferson Road - West	7	Dedicated EB left and right and 1 WB dedicated right turn lanes, 2 EB and 2 WB travel lanes	12'	continuous	3' WB, none EB	curb/bridge	

Existing Traffic Volumes and Patterns

Traffic Volumes

Existing AM and PM traffic count data for the corridor intersections were provided from various sources as noted below:

Table 3 – Summa	ary of Existing Traffic Count Dat	a
Intersection	Count Source	Date Counted
East River Road (CR 84) at:		
Erie Station Road		
Brooks Road	¹Graywood Commons TIS	November 2012
Lehigh Station Road		
Bailey Road/Chesapeake Landing	² The Grove at Rochester TIS	January 2013
River Meadow/Farnum Lane (RIT)	³ Richland Housing Development TIS	November 2014
Jefferson Road	NYSDOT	March 2009

¹Graywood Commons Development Traffic Impact Study - SRF Associates Dec 2013

The Monroe County Average Daily Traffic (ADT) within the corridor varies from a low of 4,400 vehicles (2009) at the southern study limit to a high of 11,084 vehicles (2012) between Lehigh Station and Bailey. The existing traffic count data is shown in Figure 1.

Existing Travel Patterns

The Genesee River, which parallels East River Road to the west, has played an important role in establishing local and regional mobility. Traffic patterns in the area are very much controlled by the road network and the number of Genesee River crossings. Within the study area, there are only two river crossings that allow east west travel, one at the southern project limit, NYS Route 253 (Erie Station), and the other at the northern project limit, NYS Route 252 (Jefferson Road).

Proximity to the New York State Thruway Exit 46, I-390 and West Henrietta Road (NYS 15) also influence commuter travels patterns within and adjacent to the corridor. Several east-west roads within the corridor provide connectivity to either the expressways or West Henrietta Road. These roadways include Erie Station Road, Brooks Road, Lehigh Station Road (includes interchange with I-390) and Bailey Road. Scottsville Road also provides access to I-390 which is reflective of the high number of northbound left turns at the East River Road and Jefferson Road intersection.

The Rochester Institute of Technology (RIT) is located at the north end of the study area and is currently one of the major generators/employers within the corridor. The campus currently has three (3) access points on East River as well as access from Bailey Ave via John Street. The three (3) existing East River Road access points from south to north include:

- 1. Farnum Lane/River Meadow Drive (signalized)
- 2. Andrews Memorial Drive (unsignalized)
- 3. Ward Road (unsignalized)

²The Grove at Rochester Traffic Impact Study – Passero Associates Aug 2014

³Richland Housing Development Traffic Impact Study – McFarland Johnson Nov 2014

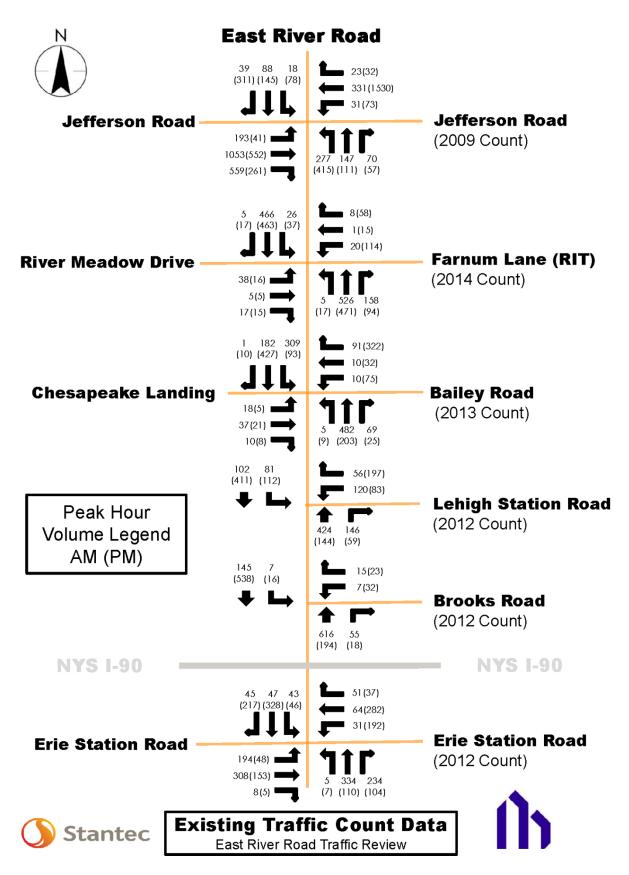


FIGURE 1 – Existing Traffic Count Data

Existing Capacity

The only intersection currently experiencing capacity issues is the East River Road and Jefferson Road intersection. The NB approach queues extend beyond the current left turn lane storage with vehicles waiting 2 or more cycles to clear the intersection. This is the only intersection with capacity issues under 2015 conditions.

FUTURE TRANSPORTATION DEMAND

Anticipated Development

Based on discussions with Town staff there are currently 20 developments that are anticipated within or adjacent to the corridor over the next 20 years. These developments include single family homes, apartments, student housing, senior housing, elementary school, light industrial and an RG&E substation. Except for the substation, all these developments will generate traffic on the adjacent roadways. The proposed development was divided into 5 year increments to provide short term and long term corridor impacts. See Table 2 for a development summary and Figure 3 for a map of the proposed East River Road developments anticipated over the next 20 years.

Area Growth

Background growth will account for anticipated corridor growth at local employers/generators like RIT as well as regional growth that originate from outside the study limits. Based on input from Monroe County, the existing traffic counts were grown by 1.5%/year in order to establish base year (2015) volumes. Based on the extensive development incorporated into this study, post 2015 growth for years 2020 thru 2035 was assumed to originate from the South (Town of Rush) with this volume grown by 0.5%/year. Future volumes with background growth are incorporated into the 'Base Volumes' for years 2015, 2020, 2025, 2030 and 2035. The Base volumes for these years also include the development generated traffic from the previous 5 years as discussed below. See Figure 2 for the background growth beyond 2015.

Committed Improvements

A widening project required by Town and constructed by a private development is planned for 2016-2017 to provide consistent shoulder widths for the roadway section just south of Fairwood Drive to Farnum Lane (RIT). Current development in construction may also introduce new driveways, shoulder work and turn lanes as required. Examples include possible shoulder work for the Kodak Riverwood project at Lehigh Station Road and the Foxfield subdivision project at Farrell Road.

Future Traffic Projections

As mentioned above the base year (2015) AM and PM peak hour traffic volumes were established by applying a 1.5% growth rate to the existing traffic count data. Background growth beyond year 2015 was established using 0.5% growth originating from the South (Town of Rush). Traffic volumes attributable to future development were established using the <u>Trip Generation Manual</u>, 9th Edition. The Trip Generation manual provides estimates of the number of trips entering and exiting a site which are generated by a specific land use. For this review we used the estimated trips generated during the peak hour (AM/PM) of the adjacent roadway. Figures 4.1 to 4.8 provides the trip generation summary for each development scenario in 5-year increments as noted above.

				East River Road Summary of Study A				
Parcel	Application	Development	Anticipated					Development Summary
No.	No.	Phase	Build-out	Name	Address	Main Access	Units	Land Use
Short Ter	m							
1	PB-221.8	Construction	1-5 Years	Section 8 - Preserve Subdivision	York Bay Trail	East River/Bailey	26	Residential - Single Family
2	PB-221.9	Construction	1-5 Years	Section 9 - Preserve Subdivision	York Bay Trail	East River/Bailey	25	Residential - Single Family
3	PB - 221.10	Design	1-5 Years	Section 10 - Preserve Subdivision	York Bay Trail	East River/Bailey	15	Residential - Single Family
4A	N/A	Pre-Concept	1-5 Years	Wallman Property	3820/3860 East River Rd	East River Road	150,000	Industrial (SF)
5	15-013	Design	1-5 Years	Riverwood Student Housing	4545 East River Road	East River Road	256	Student Housing Units (685 students)**
8	N/A	Concept	1-5 Years	Riverwood Tech Campus	East River Road	East River Road	360,000	Office and Light Industrial (SF)
_	N/A	0	1-5 Years	D-D(-laure	East River Road	Fact Division at the	100	Residential - Sr. Housing
9	N/A	Concept	1-5 Years	DePaul (Jaynes South, Parcel E)	East River Road	East River/LeHigh	400	Residential - Apartments
11	PB - 146.3	Design	1-5 Years	Section 3 - Chelsea Meadows	Thames Drive	Lehigh Station Rd	30	Residential - Single Family
13	PB - 294	Design	1-5 Years	Graywood Commons Subdivision	East River Road	East River/Brooks	200	Residential - Single Family
15	N/A	Concept	1-5 Years	Jaynes Riverview, Parcel J	East River Road	East River Road	141	Residential - Single Family
16	N/A	Concept	1-5 Years	Section 2 - Graywood Meadows (Jaynes Riverview, Parcel M)	Farrell Road Ext	East River Road	20	Residential - Single Family
18	15-008	Construction	1-5 Years	Erie Station Business Park - Flex Bldg II	30 Becker Road	Erie Station	37,851	Office (SF)
19	N/A	Concept	1-5 Years	Riverton Parcel 'A' - Phases I-IV	Erie Station Road	Erie Station Road	131	Residential - Single Family
20	PB-293	Construction	1-5 Years	Section 1&2 - Queens Park Subdivision	Moore and Martin Road	East River Road	125	Residential - Single Family
Mid-Term								
4B	N/A	Pre-Concept	5-10 Years	Wallman Property	3820/3860 East River Rd	East River Road	375	Residential - Apartments
6	N/A	Concent	5-10 Years	Jaynes North, Parcel A	East River Road	East River/LeHigh	100	Residential - Sr. Housing
ь	N/A	Concept	5-10 fears	Jaynes North, Parcel A	East River Road	East RivenLenign	400	Residential - Apartments
7	N/A	Concept	5-10 Years	Jaynes Riverview, Parcels B & D	Lehigh Station Road	Lehigh Station Rd	400	Residential - Apartments
12	N/A	Concept	5-10 Years	Jaynes Riverview, Industrial Parcel H	East River Road	East River Road	-	RG&E Substation
14	N/A	Concept	5-10 Years	Jaynes Riverview, Parcel I	Brooks Road	Brooks/East River	115	Residential - Single Family
				,				
Long-Ter	m							
10	N/A	Concept	10-15 Years	Jaynes Riverview, Parcel F	Lehigh Station Road	Lehigh Station Rd	400	Elementary School (Students)
17	N/A	Concept	10-15 Years	Jaynes Riverview, Parcel N*	East River Road	East River Road	7	Residential - Single Family

^{*} parcel is mostly wetlands

Development potential estimated in consultation with the Town; prior TIA studies for some parcels may vary.

TABLE 4 – Summary of Study Area Development

^{** 2.67} students/unit per PA Grove TIA

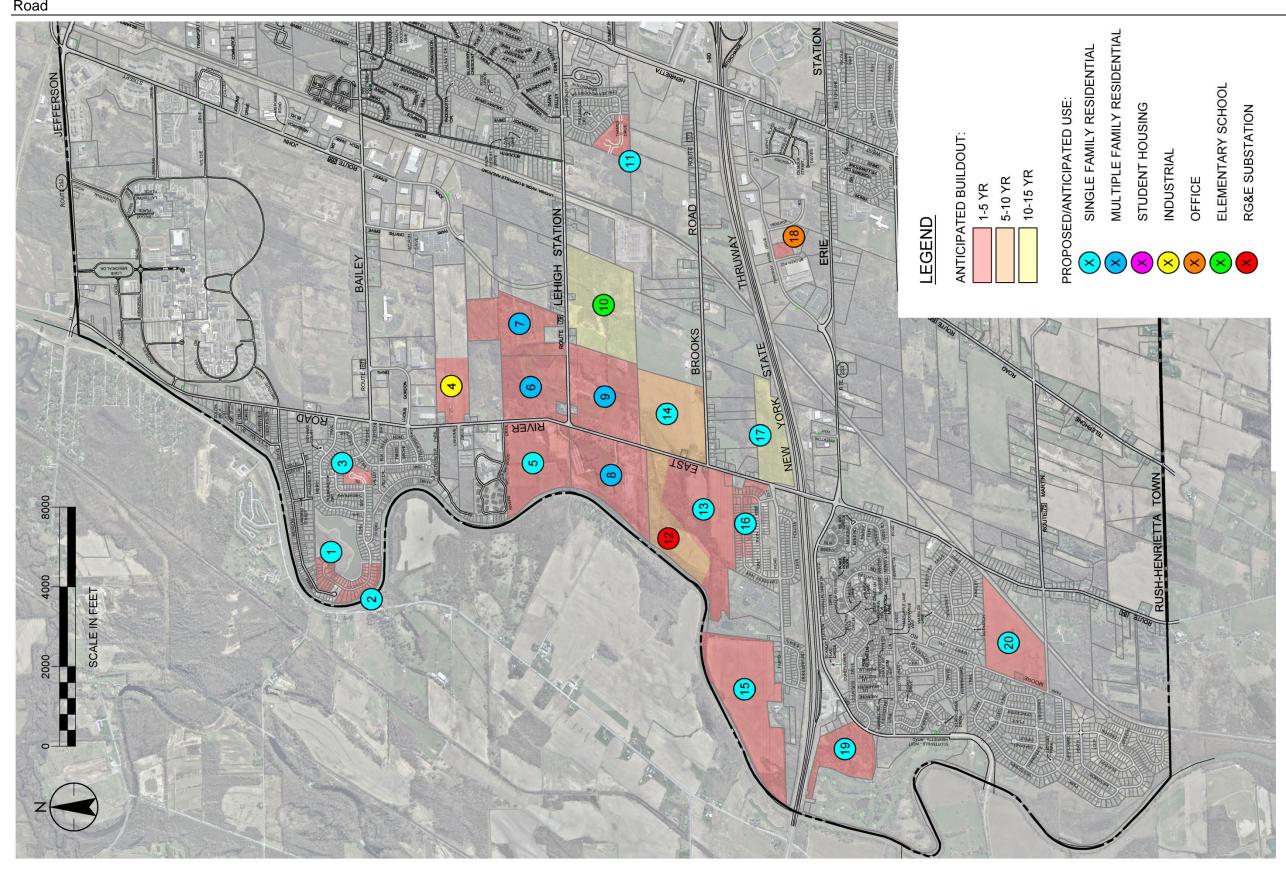


FIGURE 2 – Proposed East River Road Development

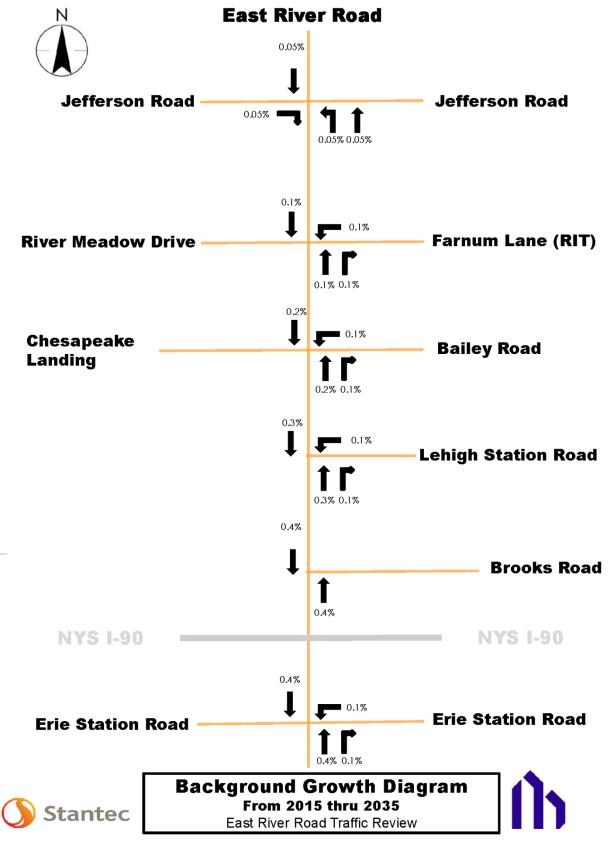


FIGURE 3 - Background Growth Years 2020 thru 2035

Future Development Trip Distribution

Assigning the future development trips to the adjacent highway network requires considering several factors including:

- Type of development
- · Location of development within corridor
- Proximity to primary commuter routes and expressways
- Existing travel patterns
- Roadway traffic volumes and capacity

Distribution diagrams were established for each development using the above factors and are included in Appendix A.

			Summary of Trip Gen	eration (9th Editio	n)						
Parcel	Name		Development Summary	Mo	rning PH			Ev	ening PH	l .	
No.		Units	Land Use	Formula	Total	Enter	Exit	Formula	Total	Enter	Exit
Short Te	rm										
1	Section 8 - Preserve Subdivision	26	Residential - Single Family	T=0.7(x)+9.74	28	7	21	Ln(T)=0.90 Ln(x)+0.51	31	20	11
2	Section 9 - Preserve Subdivision	25	Residential - Single Family	T=0.7(x)+9.74	27	7	20	Ln(T)=0.90 Ln(x)+0.51	30	19	11
3	Section 10 - Preserve Subdivision	15	Residential - Single Family	T=0.7(x)+9.74	20	5	15	Ln(T)=0.90 Ln(x)+0.51	19	12	7
4A	Wallman Property	150,000	Industrial (SF)	Ln(T)=0.79 Ln(X)+0.91	130	114	16	T=0.78(x)+30.48	148	18	130
5	Riverwood Student Housing	256	Student Housing Units (685 students)**	average per TIA (0.52)	133	27	106	average per TIA (0.60)	154	103	51
8	Riverwood Tech Campus	360,000	Office and Light Industrial (SF)	Ln(T)=0.79 Ln(X)+0.91	447	393	54	T=0.78(x)+30.48	443	67	376
9	DePaul (Jaynes South, Parcel E)	100	Residential - Sr. Housing	0.18	18	12	6	0.29	29	21	8
3	Deraul (Jaylies South, Faicel E)	400	Residential - Apartments	Ln(T)=0.82 Ln(x)+0.23	171	36	135	Ln(T)=0.88 Ln(X)+0.16	229	149	80
11	Section 3 - Chelsea Meadows	30	Residential - Single Family	T=0.7(x)+9.74	31	8	23	Ln(T)=0.90 Ln(x)+0.51	36	23	13
13	Graywood Commons Subdivision	200	Residential - Single Family	T=0.7(x)+9.74	150	37	112	Ln(T)=0.90 Ln(x)+0.51	196	123	73
15	Jaynes Riverview, Parcel J	141	Residential - Single Family	T=0.7(x)+9.74	108	27	81	Ln(T)=0.90 Ln(x)+0.51	143	90	53
16	Section 2 - Graywood Meadows (Jaynes Riverview, Parcel M)	20	Residential - Single Family	T=0.7(x)+9.74	24	6	18	Ln(T)=0.90 Ln(x)+0.51	24	15	9
18	Erie Station Business Park - Flex Bldg II	37,851	Office (SF)	Ln(T)=0.80 Ln(x) +1.57	88	77	11	T=1.12(x)+78.45	121	21	100
19	Riverton Parcel 'A' - Phases I-IV	131	Residential - Single Family	T=0.7(x)+9.74	101	25	76	Ln(T)=0.90 Ln(x)+0.51	134	84	50
20	Section 1&2 - Queens Park Subdivision	125	Residential - Single Family	T=0.7(x)+9.74	97	24	73	Ln(T)=0.90 Ln(x)+0.51	125	79	46
					1574	807	767		1861	843	1019
/lid-Term	1	T .									
4B	Wallman Property	375	Residential - Apartments	Ln(T)=0.82 Ln(x)+0.23	163	34	129	Ln(T)=0.88 Ln(X)+0.16	216	140	76
6	Jaynes North, Parcel A	100	Residential - Sr. Housing								
U	Jayries North, Farcer A	400	Residential - Apartments	Ln(T)=0.82 Ln(x)+0.23	171	36	135	Ln(T)=0.88 Ln(X)+0.16	229	149	80
7	Jaynes Riverview, Parcels B & D	400	Residential - Apartments	Ln(T)=0.82 Ln(x)+0.23	171	36	135	Ln(T)=0.88 Ln(X)+0.16	229	149	80
12	Jaynes Riverview, Industrial Parcel H		RG&E Substation	n/a	0	0	0	n/a	0	0	0
14	Jaynes Riverview, Parcel I	115	Residential - Single Family	T=0.7(x)+9.74	90	23	68	Ln(T)=0.90 Ln(x)+0.51	119	75	44
				9	595	129	467		793	513	280
.ong-Te	rm (l control of								
10	Jaynes Riverview, Parcel F	400	Elementary School (Students)	0.45	180	99	81	0.15	60	29	31
17	Jaynes Riverview, Parcel N*	7	Residential - Single Family	T=0.7(x)+9.74	15	4	11	Ln(T)=0.90 Ln(x)+0.51	10	6	4
		mg			195	103	92		70	36	34

^{*} parcel is mostly wetlands

Development potential estimated in consultation with the Town; prior TIA studies for some parcels may vary.

TABLE 5 – Trip Generation Summary

^{** 2.67} students/unit per PA Grove TIA

Full Development Volumes

The estimated full development (Full Build) future peak hour volumes were established by combining the future base volumes and estimated development generated volumes. These volumes represent an estimate of the future corridor traffic conditions at each of the six (6) analysis intersections for years 2020, 2025, 2030 and 2035. Future Traffic volumes are shown in Figures 4.2 to 4.8.

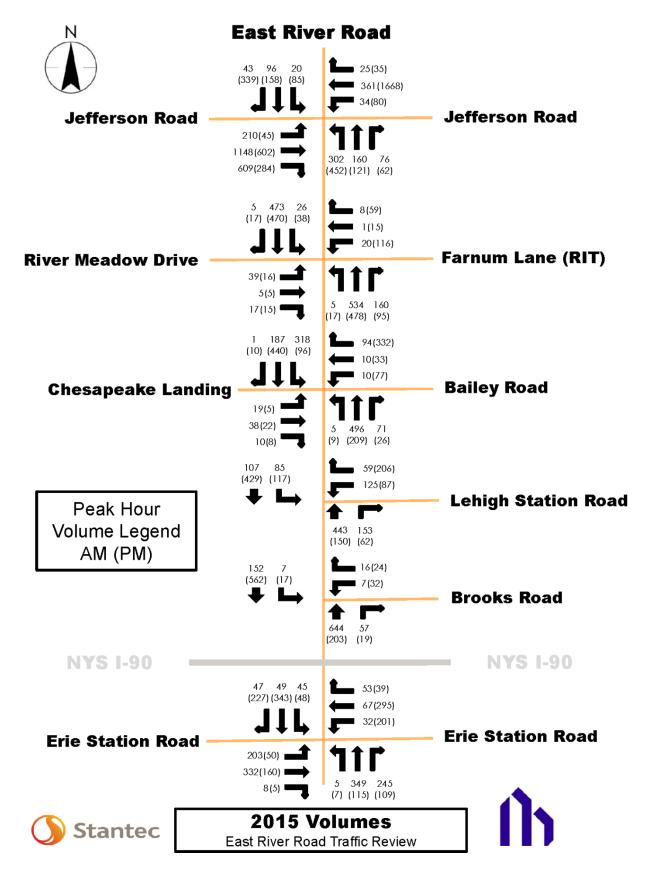


FIGURE 4.1 – 2015 Traffic Volumes

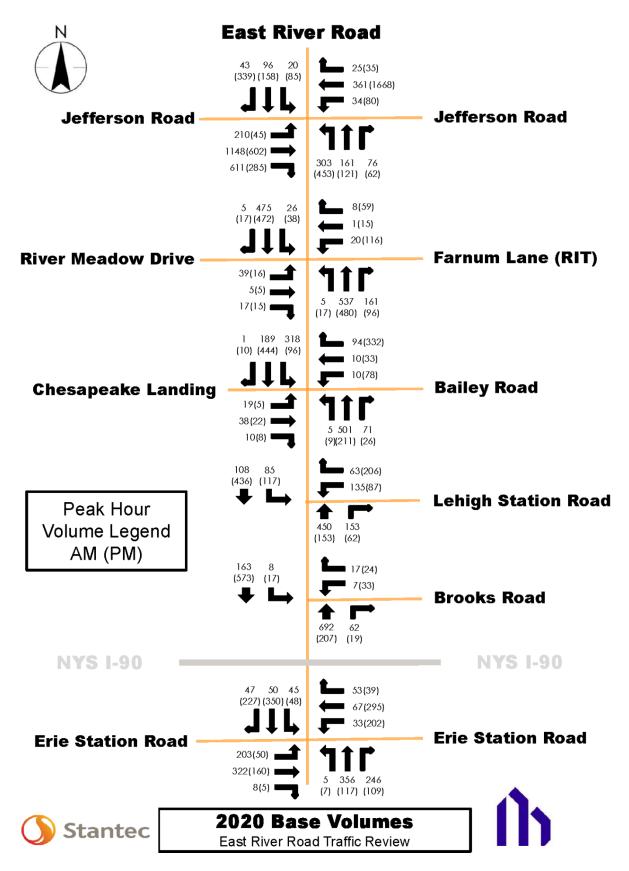


FIGURE 4.2 - 2020 Base Volumes

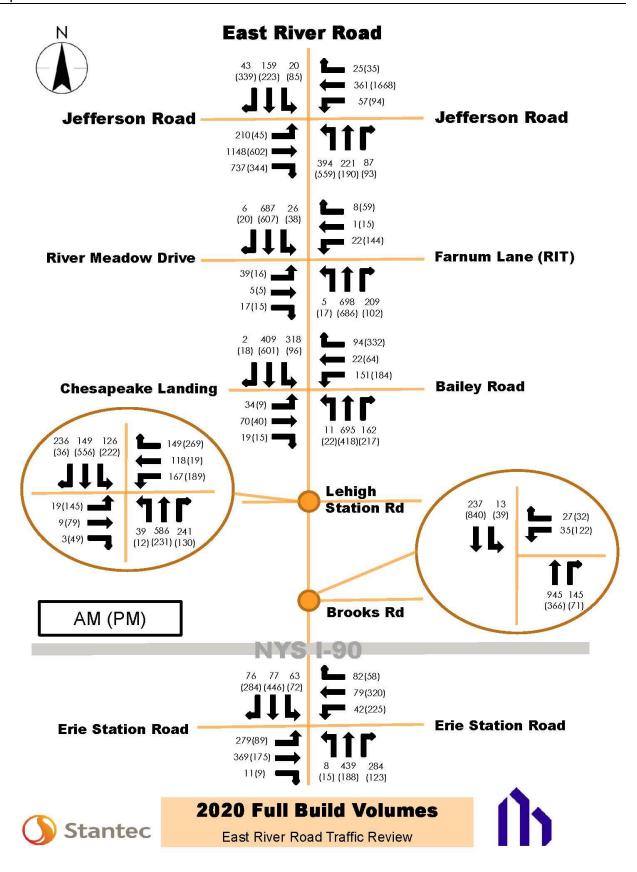


FIGURE 4.3 – 2020 Full Build Volumes

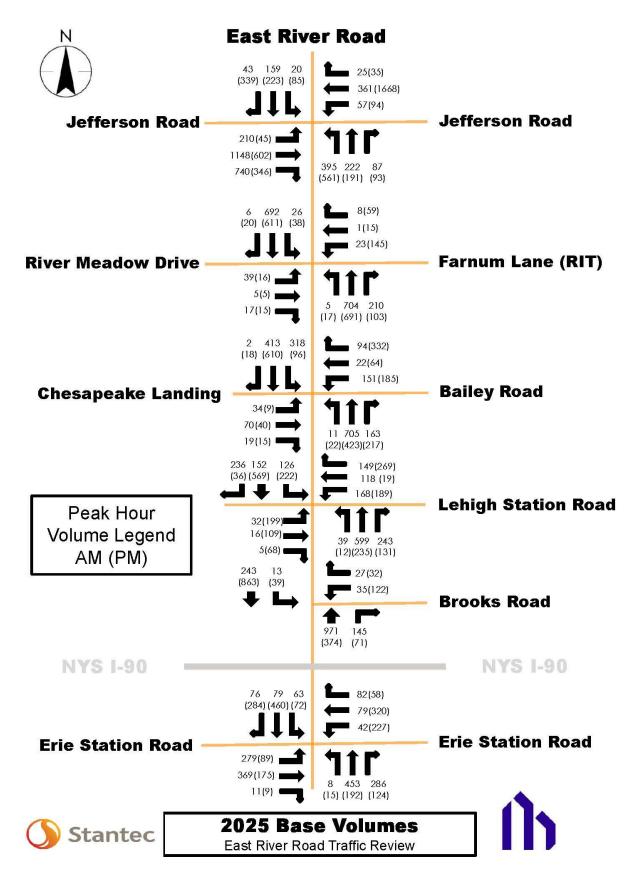


FIGURE 4.4 - 2025 Base Volumes

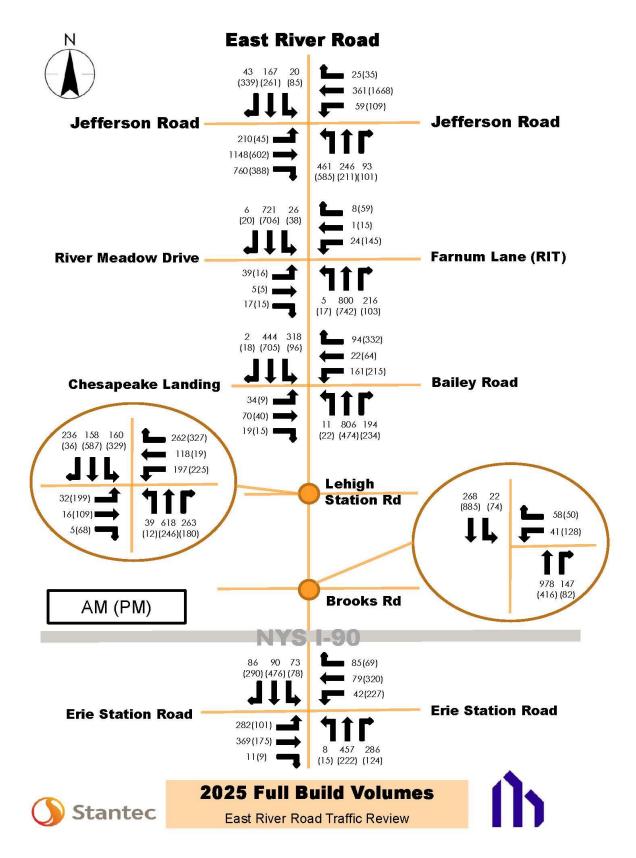


FIGURE 4.5 – 2025 Full Build Volumes

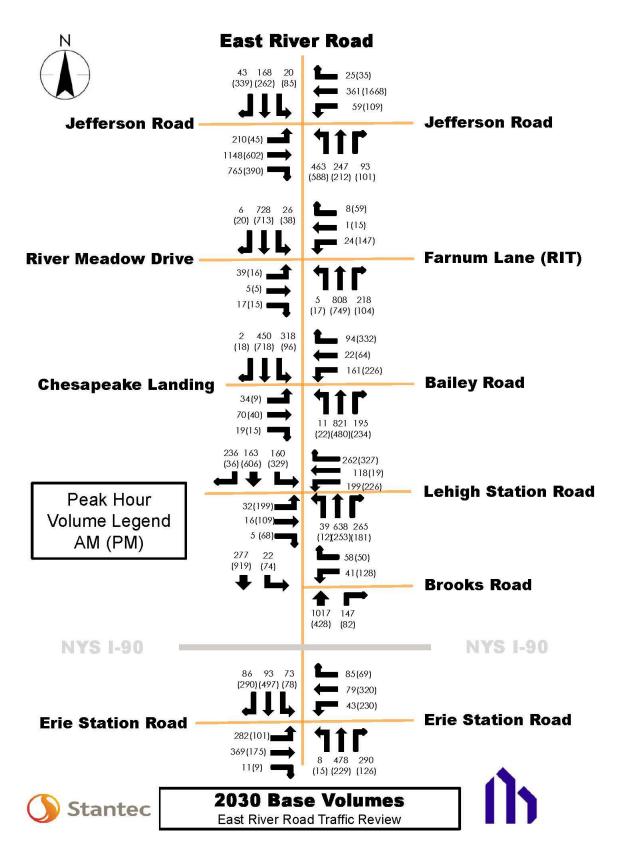


FIGURE 4.6 – 2030 Base Volumes

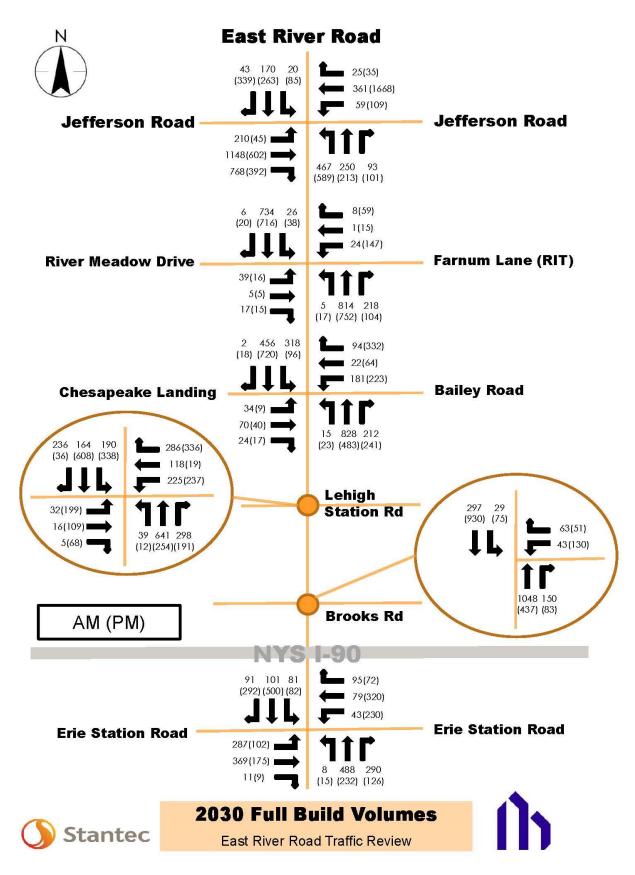


FIGURE 4.7 - 2030 Full Build Volumes

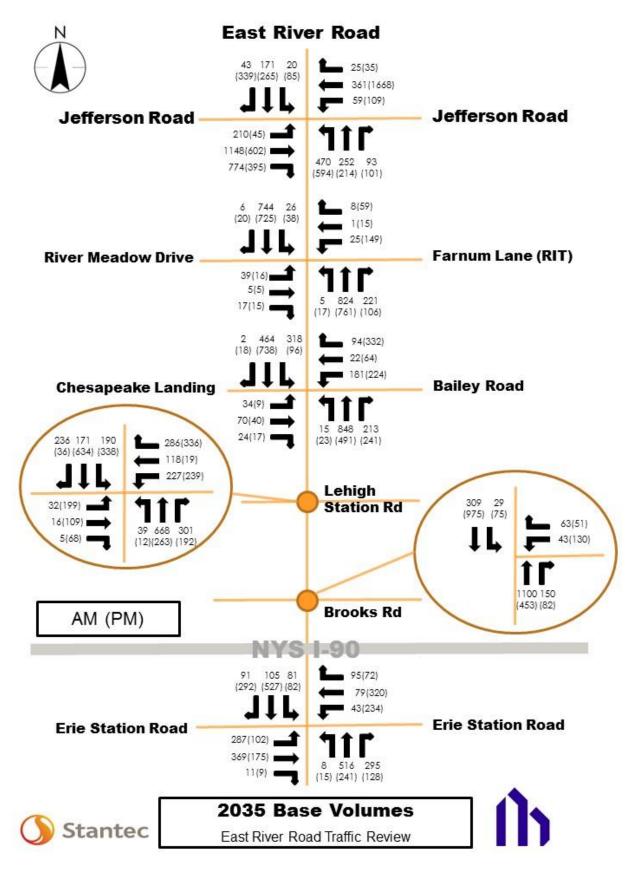


FIGURE 4.8 - 2035 Base Volumes

CAPACITY ANALYSIS

Intersection Capacity Analysis – Unsignalized and Signalized Intersections

The Highway Capacity Manual (HCM) defines capacity as; "The maximum sustainable flow rate at which vehicles or persons reasonably can be expected to traverse a point or uniform segment of a lane or roadway during a specified time period under given roadway, geometric, traffic, environmental, and control condition." Level of service (LOS) is a qualitative measure used to relate the quality of traffic service. LOS is used to analyze highways by categorizing traffic flow and assigning quality levels of traffic based on performance measure like speed, density, etc.

The HCM defines LOS for signalized and unsignalized intersections as a function of the average vehicle control delay. LOS may be calculated per movement or per approach for any intersection configuration, but LOS for the intersection as a whole is only defined for signalized and all-way stop configurations. Table 6 below provides the HCM LOS Criteria.

	Table 6 – HCM 2010) LOS Criteria
LOS	Signalized Intersection Controlled Delay (sec/veh)	Unsignalized Intersection Controlled Delay (sec/veh)
Α	≤10 sec	≤10 sec
В	>10 and ≤20 sec	>10 and ≤15 sec
С	>20 and ≤35 sec	>15 and ≤25 sec
D	>35 and ≤55 sec	>25 and ≤35 sec
Е	>55 and ≤80 sec	>35 and ≤50 sec
F	>80 sec	>50 sec

The Monroe County Department of Transportation considers the following to be the minimum Level of Service expectations at signalized intersections.

- The LOS shall be "D" or better for the overall intersection and for each of its individual approaches, AND
- The LOS shall be "E" or better on every individual movement, AND
- The v/c ratios shall be less than 1.00 for every individual movement.

A capacity analysis was performed at each of the six (6) intersections using the traffic modeling software Synchro Version 9.0, which utilizes HCM 2010 methodologies. Level of Service was calculated for each of the following analysis scenarios:

- 2015 (Existing)
- 2020/2025/2030 Base Conditions
- 2020 /2025/2030/2035 Full Development Conditions

Base conditions for years 2025/2030 builds upon previous full development conditions. No development was included beyond year 2030. The capacity analysis results are shown in Figures 5.1 to 5.5 - LOS table. Synchro analysis printouts are contained in Appendix B.

April 2018									IDOR I		C ANAL	<u>- 1010 f</u>									
	 		AM Peak	Hour				P	M Peak Ho		1		201	5 AM Mitig				201	5 PM Mitig		1
	LANE	V/C		Level of	50th	95th	V/C	l	Level of	50th	95th	V/C	l ₋ .	Level of	50th	95th	V/C		Level of	50th	95th
	GROUP	Ratio	Delay	Service	Queue	Queue	Ratio	Delay	Service	Queue	Queue	Ratio	Delay	Service	Queue	Queue	Ratio	Delay	Service	Queue	Queue
Intersection					(ft)	(ft)				(ft)	(ft)				(ft)	(ft)				(ft)	(ft)
F Divor Dal 9	EB-LTR	0.52	19.7	В	120	169	0.21	15.6	В	40	65										
E River Rd & 253	WB-LTR NB-LTR	0.15 0.87	9.9 35.6	A D	17 297	36 510	0.53 0.34	19.6 13.8	B B	116 65	167 122										
Signalized	SB-LT	0.87	17.7	В	35	70	0.60	23.6	C	175	272		١	No Mitigatio	on			1	lo Mitigati	on	
Signanzea	SB-R	0.07	3.6	A	0	16	0.31	3.3	A	0	42										
	Intersection	-	24.7	С	-	-	-	16.9	В	-	-										
E River Rd &	WB-LR	0.07	15.6	Α	-	6	0.15	15.2	В	-	13										
Brooks	NB-TR	0.46	0.0	Α	-	0	0.15	0.0	Α	-	0		N	No Mitigatio	∩n			N	lo Mitigati	on	
Unsignalized	SB-LT	0.01	0.5	A	-	1	0.01	0.4	Α	-	1										
	Intersection		0.5	Α	-	-	-	1.3	А	-	-										
	WB-L	0.42	31.2	С	58	108	0.19	13.1	В	15	51										
E River Rd &	WB-R	0.19	8.6	Α	0	30	0.37	4.6	Α	0	39										
Lehigh Station	NB-T	0.35	5.6	Α	77	152	0.18	6.5	A	19	45										
Signalized	NB-R	0.14	3.1	Α	13	38	0.08	2.2	Α	0	12		N	No Mitigatio	on			ľ	lo Mitigati	on	
	SB-L	0.15	5.2	Α	12	34	0.22	7.2	Α	15	39										
	SB-T	0.09	4.3	Α	15	36	0.52	9.6	А	65	135										
	Intersection	-	8.5	Α	-	-	-	7.8	А	-	-										
	EB-L	0.01	18.0	Α	-	1	0.12	10.1	В	-	0.4										
	EB-R	0.01	9.4	Α	-	1	0.15	9.0	Α	-	0.5										
E RIVER Rd &	NB-L	0.10	8.1	Α	-	8	0.01	7.9	А	-	0.0										
Kodak N	NB-T	0.25	0.0	Α	-	0	0.60	15.5	В	-	4.1		N	No Mitigatio	on			ı	lo Mitigati	on	
Unsignalized	SB-T	0.12	0.0	Α	-	0	0.79	24.2	С	-	7.4										
	SB-R	0.04	0.0	Α	-	0	0.01	6.6	Α	-	0.0										
	Intersection	-	1.3	Α	-	-	-	18.8	В	-	-										
	EB-LT	0.21	31.2	С	26	65	0.06	16.1	В	7	25										
	EB-R	0.03	0.2	Α	0	0	0.02	0.1	Α	0	0										
	WB-LT	0.08	29.9	С	9	31	0.31	19.3	В	30	75										
E River Rd &	WB-R	0.28	9.3	Α	0	42	0.53	5.6	Α	0	54										
Chesapeake Landing/ Bailey	NB-L	0.01	12.2	В	1	9	0.03	12.9	В	2	11		١	No Mitigatio	on			ı	lo Mitigati	on	
Signalized	NB-TR	0.72	21.7	С	211	404	0.43	16.1	В	57	123										
	SB-L	0.55	7.9	Α	37	106	0.14	4.6	Α	10	29										
	SB-TR	0.14	3.2	Α	23	42	0.49	8.4	Α	66	151										
	Intersection	-	14.9	В	-	-	-	9.9	Α	-	-										
	EB-LTR	0.19	17.6	В	15	46	0.11	13.0	В	5	29										
E River Rd &	WB-LT	0.06	20.9	С	7	24	0.43	22.3	С	34	99										
Meadows / RIT	WB-R	0.02	0.1	А	0	0	0.15	6.6	А	0	26		N.	la Mitianti	n n			N	lo Mitigati	on	
Signalized	NB-LTR	0.54	6.2	Α	126	220	0.55	9.3	Α	104	247		ľ	No Mitigation	UII			1	io iviitigati	OII	
	SB-LTR	0.40	4.9	Α	80	136	0.51	8.9	Α	92	215										
	Intersection	-	6.4	Α	-	-	-	10.4	В	-	-										
	EB-L	0.41	10.4	В	54	79	0.29	30.6	С	27	50	0.40	4.9	Α	30	49	0.20	5.6	Α	4	7
	EB-T	0.62	15.4	В	367	383	0.36	30.6	C	194	238	0.61	6.9	A	174	240	0.31	6	A	72	98
Epine DIO	EB-R	0.56	3.4	A	33	36	0.31	16.3	В	118	167	0.55	1.7	A	12	23	0.28	1.9	A	6	7
E River Rd &	WB-L	0.26	24.7	С	15	45	0.27	30.4	С	45	96	0.25	23.9	С	15	44	0.24	19.9	В	31	69
Jefferson	WB-TR	0.20	16.5	В	57	84	0.87	39.8	D	495	677	0.20	15.9	В	56	84	0.80	26.6	C	347	429
Signalized	NB-L	0.84	48.2	D	184	269	0.82	39.5	D	272	349	0.67	47.1	D	106	148	0.82	53.5	D	148	220
	NB-T	0.31	26.9	С	87	132	0.15	19.9	В	57	89	0.33	27.7	С	88	132	0.21	24.8	С	56	100
	NB-R	0.16	7.3	Α	4	34	0.09	4.2	Α	0	24	0.16	7.4	Α	4	34	0.11	4	Α	0	20
	SB-L	0.17	41.0	D	13	36	0.45	54.1	D	64	119	0.17	41.0	D	13	36	0.59	58.1	E	53	112
	SB-T	0.50	48.7	D	66	113	0.55	54.6	D	120	194	0.50	48.7	D	66	113	0.71	60.1	Е	101	185
	SB-R	0.16	1.2	Α	0	0	0.86	44.8	D	134	293	0.10	3.0	Α	0	12	0.75	35.8	D	152	251
	Intersection		17.5	В	-	-	-	36.5	D	-	-	-	13.5	В	-	-	-	26.8	С	-	-
							•	_		-				_	•	_	_			_	

April 2018											С	ORR	DOR	TRA	FFIC .			REPO	ORT												
			A	M Peak Ho	our	2020	Base	PM	/ Peak Ho	ur			A	M Peak H	lour	2020	Build	PN	/I Peak Ho	ur			A	M Peak Ho		2020 Build	d Mitigation		M Peak Ho	ur	
Intersection	LA NE GROUP	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)
	EB-LTR	0.51	19.6	В	117	166	0.33	17.1	В	53	84	0.90	44.3	D	195	305	0.43	23.3	С	63	100	0.80	32.0	С	172	295	0.40	20.3	С	51	102
E River Rd & 253	WB-LTR NB-LTR	0.15 0.89	9.9 37.0	A D	17 304	37 523	0.60 0.48	21.0 18.2	C B	134 112	191 188	0.30 0.84	14.3 25.3	B C	27 322	56 572	0.76 0.41	30.6 13.1	C B	161 98	228 163	0.25	12.8 24.9	B C	24 322	55 501	0.74 0.46	26.4 14	C B	132 90	234 175
Signalized	SB-LT	0.24	17.8	В	36	72	0.79	31.7	С	247	419	0.27	12.3	В	42	80	0.69	21.6	С	223	346	0.26	11.6	В	42	77	0.76	24.5	С	204	367
· ·	SB-R	0.07	3.6	Α	0	16	0.37	3.4	Α	0	46	0.09	2.7	Α	0	20	0.34	3.2	Α	7	47	0.10	2.5	Α	0	19	0.36	3.2	А	3	45
E River Rd &	Intersection WB-LR	0.08	25.3 16.5	A	-	- 6	0.46	20.2 35.3	D	-	- 56	0.39	37.4 37.5	D E	-	42	1.06	20.5 145.5	C F	-	217	-	24.1 Movem	ent Does f	- VotExist	-	-	19.8 Movem	B nent Does N	Vot Exist	-
Brooks	WB-L			ient Does l	Not Exist				ent Does N	lot Exist				nent Does	Not Exist				ent Does f	Vot Exist		0.24	33.7	D	-	22	0.95	123.1	F	-	168
Unsignalized	WB-R	0.10			T		0.07			JOELAISE		0.74			T		0.20			I		0.11	19.6	C ent Does f	- Vat Eviat	9	0.06	10.9	B nent Does N	-	4
	NB-TR NB-T	0.49	0.0 Movem	A ient Does l	Not Exist	0	0.27	0.0 Moveme	A ent Does N	lot Exist	0	0.71	0.0 Movem	A nent Does	Not Exist	0	0.29	0.0 Movemi	A ent Does l	Not Exist	0	0.62	0.0	A	-	0	0.24	0.0	A A	-	0
	NB-R SB-LT	0.01	0.6	A	I -	1	0.04	1.0	А	_	3	0.02	0.9	A	Ι	2	0.04	1.1	А	I .	3	0.09	0.0	A ent Does f	- Vot Evist	0	0.05	0.0	A nent Does N	- Not Evist	0
	SB-L	0.01		ent Does l		±	0.04		ent Does N	JotExist	,	0.02		nent Does	Not Exist		0.04		ent Does f	Not Exist		0.02	11.4	В	-	2	0.04	8.5	А	-	3
	SB-T Intersection	-	0.5	A	-	-	-	2.9	А	-	-	-	1.8	A	-	-	-	16	С	-	-	0.15	0.0 1.3	A	-	-	0.55	0.0 10.7	A B	-	-
	EB-L		Movem	ent Does f	Not Exist			Moveme	ent Does N	JotExist		0.33	34.2	С	15	42	0.69	31.9	С	72	186	0.24	29.0	С	14	40	0.61	28.4	С	70	213
	EB-TR WB-L	0.45	31.7	С	63	116	0.19	13.2	В	15	51	0.07 0.59	19.7 34.2	B C	7 78	25 140	0.28 0.46	13.8 20.4	B C	41 60	105 146	0.05	18.7 30.2	В	7 78	24 140	0.27 0.44	14.4 21.3	B C	42 61	114 157
	WB-LT			ent Does (ent Does N					nent Does		_ :-			ent Does (ent Does f					nent Does N		
E River Rd &	WB-R	0.20	8.4	А	0	31	0.37	4.6	Α	0	39	0.55				470	0.00				F.C.	0.50				4.70					T 60
Lehigh Station Signalized	WB-TR NB-T	0.39	6.4	А	81	157	0.18	6.5	А	19	45	0.66	26.7	C nent Does	91 Not Exist	170	0.39	4.3	A ent Does f	5 Not Exist	56	0.60	23.4	C ent Does f	91	170	0.39	4.7	A nent Does N	5 Not Evist	62
	NB-LT NB-L		Movem	ient Does f	Not Exist			Moveme	ent Does N	Jot Exist		0.06	6.3	I A	7	21	0.06	12.5	R R	2	14	0.06	5.6	A A	6	19	0.07	10.1	R B	2	11
	NB-TR											0.75	13.3	В	223	466	0.47	13.6	В	90	198	0.80	15.5	В	223	466	0.48	12.4	В	92	152
	NB-R	0.15	3.3 5.4	A	14	38	0.08	2.2	A	0	12	0.60		nent Does	_	1.47	0.65		ent Does f		100	0.00		ent Does f		1.54	0.50		nent Does N		1.50
	SB-L SB-TR	0.17		A ient Does î	13 Not Exist	35	0.22	7.2 Moveme	A ent Does N	15 Jot Exist	40	0.62	25 Movem	C nent Does	32 Not Exist	147	0.65	24.4 Movem	C ent Does î	71 Not Exist	186	0.80	47.9 Movem	D ent Does f	37 Vot Exist	164	0.68	24.7 Movem	C nent Does N	76 Not Exist	152
	SB-T	0.09	4.5	Α	15	37	0.52	9.7	Α	66	137	0.13	5.2	Α	23	52	0.70	20.2	С	185	376	0.14	5.4	Α	23	52	0.72	19.9	В	196	297
	SB-R Intersection		9.2	А				7.8	А			0.24	1.5 16.1	A B	0	27	0.05	4.9 17.5	A B	1	17	0.25	1.7 17.6	A B	0	27	0.05	3.3 17.1	A B	0	13
	EB-L	0.01	18.3	A	-	1	0.12	10.1	A	-	0.4		10.1	В	-	-	•	17.3	В	-	-	-	17.0	<u> </u>	•	-	-	17.1	В	•	
E RIVER Rd &	EB-R NB-L	0.01	9.4 8.1	A A	- :	1 ο	0.15 0.01	9.0 7.9	A A	-	0.5 0.0																				
Kodak N	NB-T	0.26	0.0	A	-	0	0.61	15.7	A	-	4.2		Intersed	tion Does	s Not Exist			Intersect	tion Does	Not Exist			Intersec	tion Does	Not Exist			Intersec	tion Does	NotExist	
Unsignalized	SB-T	0.12	0.0	A	-	0	0.80	25.3	A	-	7.7																				
	SB-R Intersection	0.04	0.0 1.3	A A	-	-	0.01	6.6 19.4	A A	-	0. 0 -																				
	EB-LT	0.19	28.3	C	22	64	0.06	16.3	В	7	25	0.50	38.1	D	53	105	0.12	23.4	C	21	48	0.50	38.1	D	53	105	0.12	23.4	С	21	48
	EB-R WB-LT	0.03	0.1 27.5	A C	7	30	0.02 0.31	0.1 19.4	A B	30 30	0 76	0.06 0.91	0.3 79.1	A E	95	0 217	0.03 0.74	0.1 41.0	A D	0 126	0 242	0.06 0.91	0.3 79.1	A E	0 95	0 217	0.03 0.74	0.1 41.0	A D	0 126	0 242
E River Rd &	WB-R	0.26	8.9	А	0	42	0.53	5.6	А	0	55	0.29	8.8	А	0	40	0.53	6.2	А	0	63	0.29	8.8	А	0	40	0.53	6.2	А	0	63
Chesapeake Landing/Bailey	NB-L	0.01	9.6 19.4	A B	1	7	0.03 0.43	12.9 16.1	В	2 57	11	0.02 0.98	9.2 44.3	A	3 427	10 714	0.07 0.83	13.1	В	7	20 499	0.02	9.2 44.3	A	3 427	10 714	0.07 0.83	13.1	В	7 283	20 499
Signalized	NB-TR NB-T	0.73			185	353	0.43		В		123	0.98		D		/14	0.03	28.0	С	283	499	0.98		D		714	0.83	28.0	C		499
	NB-R			ent Does (,				ent Does N					ent Does					ent Does f					ent Does f					nent Does N		
	SB-L SB-TR	0.58 0.15	9.5 3.4	A A	37 23	109 41	0.14	4.6 8.5	A	10 67	29 154	0.94 0.34	56.5 5.0	E A	121 69	284 107	0.25 0.61	6.5 11.3	A B	17 177	34 272	0.94	56.5 5.0	E A	121 69	284 107	0.25 0.61	6.5 11.3	A B	17 177	34 272
	Intersection	-	14.0	В	-	-	-	10.0	A	-	-	-	38.6	D	-	-	-	19.4	В	-	-	-	38.6	D	-	-	- 0.01	19.4	В	-	-
E River Rd &	EB-LTR	0.19	17 20.2	В	15 7	44 23	0.11	13.7	В	5	30 106	0.21	18.3 21.3	В	18	45 25	0.11 0.57	15.2	В	8	30										
E RIVER RO & Meadows / RIT	WB-LT WB-R	0.06 0.02	0.1	C A	0	0	0.44	23.4 6.9	C A	35 0	27	0.07	0.1	C A	9	0	0.57	31 7.1	C A	67 0	127 27	-									
Signalized	NB-LTR	0.54	6.2	А	122	223	0.55	9.1	Α	103	254	0.69	9.6	Α	210	516	0.77	15.5	В	220	489		N	lo Mitigatio	on			No	lo Mitigatio	on	
	SB-LTR Intersection	0.40	5.0 6.4	A A	80	140	0.51	8.8 10.4	A B	91	223	0.56 -	6.6 8.8	A	142	247	0.67	12.2 15.4	B B	164	341										
	EB-L	0.40	4.9	A	30	49	0.20	5.6	Α	4	7	0.54	16.6	В	74	71	0.24	13.4	В	25	27										
	EB-T EB-R	0.61 0.55	6.9 1.7	A A	174 12	240 23	0.31 0.28	6.0 1.9	A	74 6	100 7	0.79 0.75	24 6.6	C A	361 67	361 61	0.33 0.34	15.8 8.8	B A	190 137	241 195										
E River Rd &	WB-L	0.25	23.9	С	15	44	0.24	19.9	В	31	69	0.82	90.7	F	36	110	0.30	25.8	С	47	98										
Jefferson Signalized	WB-TR	0.20 0.67	15.9 47.1	B D	56 107	84 148	0.80	26.6 53.6	C D	347 149	429 221	0.24 0.76	19.3 48.7	B D	64 138	78 191	0.83 0.87	33.7 61.1	C E	430 223	535 307										
эідпангеа	NB-L NB-T	0.87	27.7	С	89	133	0.83	24.8	С	56	100	0.76	21.2	С	104	174	0.87	26.7	C	102	160		N	lo Mitigatio	on			N	lo Mitigatio	on	
	NB-R	0.16	7.4	A	4	34	0.11	4.0	A	0	20	0.14	7.4	Α	8	40	0.15	5.1	A	0	34										
	SB-L SB-T	0.17 0.50	41.1 48.7	D D	13 66	36 113	0.59 0.71	58.1 60.1	E E	53 101	112 185	0.09 0.42	35.1 38.5	D D	12 102	35 172	0.51 0.82	58 72.8	E	63 173	119 293										
	SB-R	0.10	3.0	A	0	12	0.75	35.8	D	152	251	0.08	3	A	0	13	0.75	42.7	D	200	298										
	Intersection	-	13.5	В	-	-	-	26.8	С		-	-	23.1	С	-	-	-	34.7	С	-	-										

Third part Thi	PM Peak Hou	ur	
Intersection Column Colu	r cak riot		
Martine Mart		50th	95th
E River M & WELL N. 196 1.00	Delay Level of	Queue	1
River Rd & Me-LTR 0.36 1.32 B 24 55 0.76 28.8 C 137 245 0.76 28.5 C 149 245	Service	(ft)	(ft)
Pin-Art 1999 2-4.9 C 225 509 0.45 13.3 D 2-7			
Set			
Se-Re 0.09 2.4 A 0 0.8 0.38 0.37 A 9 0.51 0.11 2.3 A 0 2.0 0.38 5.5 A 8 3.0	No Mitigatio	on	
Merene No.			
Filter Rd & Wilstan Novement Decay No. Filter No.			
West Color			
Mar-R Movement Color Not Dist Movemen			
No Fig. No F			
MB-T 0.63 0			
NB-R 0.09 0			
Set Movement Does Not Edit Set S	No Mitigatio	on	
SB-L 0.02 11.6 B - 2 0.04 8.9 A - 3 0.04 11.8 B - 3 0.08 8.9 A - 7			
Set			
Intersection			
E River Rd & Lehigh Station Signalized BB-I			
ERIVER R 6. 05 18.5 8 7 24 0.29 14.0 8 41 105 0.05 18.0 8 7 24 0.30 17.7 8 62 115 WB-1			
Fraction			
Lehigh Station WB-TR 0.62 24.6 C 91 170 0.41 4.6 A 5 57 0.81 32.0 C 130 266 0.47 5.1 A 8 66 66 67 68 68 68 68			
WB-TR 0.62 2.46 C 91 170 0.41 4.6 A 5 57 0.81 32.0 C 130 2.66 0.47 5.1 A B 66			
NB-L 0.06 5.5 A 6 19 0.06 12.4 B 3 14 0.06 6.0 A 7 19 0.05 10.7 B 3 13 14 15 15 15 15 15 15 15			
NB-L NB-T NB-TR			
NB-TR NB-			
NB-TR 0.79 15.2 B 232 486 0.47 13.4 B 90 202 0.82 17.6 B 325 640 0.49 13 B 135 214 NB-R Movement Does Not Exist Movement Do	No Mitigatio	on	
NB-R Movement Does Not Exist			
SB-L 0.77 42.1 D 37 163 0.64 24.1 C 70 187 1.10 123.3 F 106 151 0.95 57.6 E 183 376 SB-T 0.14 5.4 A 24 53 0.71 20.2 C 183 389 0.14 5.8 A 31 55 0.66 18.5 B 247 366 SB-R 0.25 1.6 A 0 27 0.05 4.9 A 1 17 0.24 1.6 A 0 247 366 38 247 366 38 247 366 38 38 0.14 5.8 A 1.6 A 0 27 0.05 4.0 A 1 15 15 15 0.66 18.5 B 247 366 38 38 0.14 2.0 2.7 2.0 0.05 4.0 1.0 2.7			
SB-T 0.14 5.4 A 24 53 0.71 20.2 C 188 389 0.14 5.8 A 31 55 0.66 18.5 B 247 366 SB-R 0.25 1.6 A 0 27 0.05 4.9 A 1 17 0.24 1.6 A 0 27 0.05 4.0 A 1 15 11 15 15 15 16 A 0 27 0.05 4.9 A 1 17 0.24 1.6 A 0 27 0.05 4.0 A 1 15 15 15 15 15 15 16 1 2 2 2 1			
SB-R 0.25 1.6 A 0 27 0.05 4.9 A 1 17 0.24 1.6 A 0 27 0.05 4.9 A 1 17 0.24 1.6 A 0 27 0.05 4.0 A 1 15 Intersection - 17.4 B - - - 17.6 B - - - 27.3 C - - 26.7 C - <th></th> <th></th> <th></th>			
Intersection Fig.			
EB-LT 0.50 38.1 D 53 105 0.12 23.4 C 21 48 0.57 43.1 D 54 116 0.12 24.2 C 21 49 0.42 36.7 D 58 110 0.10 EB-R 0.06 0.3 A 0 0 0 0.03 0.1 A 0 0 0 0.00 0.03 0.1 A 0 0 0 0.00 0.00 0.00 0.00 0.00 0.			
EB-R 0.06 0.3 A 0 0 0.03 0.1 A 0 0 0.03 0.1 A 0 0 0.03 0.1 A 0 0 0.03 0 0.03 0.1 A 0 0 0.03 0 0.03 0.1 A 0 0 0.03 A 0 0 0.03 0 0.03 0.1 A 0 0 0.03 0 0.03 0.1 A 0 0 0.03 A 0 0 0.03 0 0.03 0.1 A 0 0 0.03 0 0.03 0 0 0.03 0 0 0 0 0.03 0 <th>22.0 C</th> <th>19</th> <th>49</th>	22.0 C	19	49
WB-LT 0.91 79.1 E 95 217 0.75 41.5 D 127 246 1.01 104.6 F 105 236 0.86 53.1 D 150 298 0.86 69.1 E 112 236 0.73 E River Rd & WB-R 0.29 8.8 A 0 40 0.53 6.1 A 0 63 0.30 9.1 A 0 41 0.50 Chesapeake Landing/ Bailey NB-L 0.02 9.2 A 3 10 0.07 13.1 B 7 20 0.02 8.5 A 0 6.3 A 0 6.3 A 0 64 0.26 8.5 A 0 41 0.50	0.1 A	0	0
E River Rd & WB-R 0.29 8.8 A 0 40 0.53 6.1 A 0 63 0.30 9.1 A 0 41 0.54 6.3 A 0 64 0.26 8.5 A 0 41 0.50 Chesapeake Landing/Bailey NB-L 0.02 9.2 A 3 10 0.07 13.1 B 7 20 0.02 8.5 A 3 10 0.09 13.0 B 6 20 0.03 10.5 B 3 12 0.12	37.3 D	139	275
	5.4 A	0	61
Signalized NB-T Movement Does Not Exist 0.92 35.8 D 438 710 0.66	16.9 B	8	25
	23.4 C	216	338
NB-TR 0.99 47.3 D 440 728 0.84 28.5 C 289 507 1.10 81.7 F 639 877 0.90 34.3 C 338 584 Movement Does Not Exist	Movement Does N	Not Exist	
NB-R Movement Does Not Exist 0.23 2.3 A 0 32 0.31	3.3 A	0	42
SB-L 0.94 56.5 E 121 284 0.25 6.5 A 17 34 1.01 72.9 E 125 297 0.28 6.7 A 16 33 0.98 69.0 E 147 326 0.21	7.9 A	21	43
SB-TR 0.34 5.0 A 70 109 0.62 11.5 B 182 279 0.37 5.0 A 75 115 0.70 13.1 B 221 341 0.38 6.2 A 96 143 0.76	18.1 B	278	447
Intersection - 39.8 D 19.6 B 60.6 E 23.7 C 32.8 C	17.7 B	-	-
EB-LTR 0.22 22.1 C 21 52 0.11 15.4 B 8 31 0.24 22.5 C 21 52 0.12 15.3 B 8 31			
WB-LT 0.09 26.2 C 11 30 0.57 31.2 C 69 129 0.10 26.4 C 12 31 0.58 32 C 69 129			
WB-R 0.03 0.1 A 0 0 0.16 7.2 A 0 27 0.03 0.1 A 0 0 0.16 7.2 A 0 27 0.03 0.1 A 0 0 0 0.16 7.2 A 0 27 0.03 0.1 A 0 0 0 0.16 7.2 A 0.16 7.2 A 0 0 0.16 7.2 A 0 0.16 7.2 A 0 0.16 7.2 A 0 0 0			
NB-LTR 0.68 8 A 214 384 0.78 15.6 B 226 480 0.75 10.7 B 281 664 0.82 17.8 B 257 616 E River Rd & NB-L			
Iviovement Does Not Exist	No Mitigatio	on	
SB-L SB-LTR 0.55 5.8 A 143 237 0.68 12.3 B 167 339 0.57 6.1 A 155 259 0.76 15.3 B 213 451			
SB-LTR 0.55 5.8 A 143 237 0.68 12.3 B 167 339 0.57 6.1 A 155 259 0.76 15.3 B 213 451 SB-TR Movement Does Not Exist			
Intersection			
EB-L 0.44 5.9 A 43 45 0.23 6.8 A 6 10 0.46 6.8 A 50 51 0.23 7.6 A 8 14 0.41 11.4 B 58 66 0.19	8.8 A	4	10
EB-T 0.64 8.3 A 226 215 0.33 7.9 A 104 141 0.56 9.4 A 230 228 0.35 9.3 A 136 175 0.63 15 B 198 313 0.31	5.2 A	27	57
EB-R 0.69 3.6 A 46 37 0.35 3.7 A 35 47 0.71 4.2 A 59 48 0.39 4.9 A 82 158 0.72 7.4 A 90 87 0.37	1.3 A	0	0
E River Rd & WB-L 0.46 30.7 C 27 73 0.31 20.6 C 35 75 0.52 37.2 D 29 89 0.38 23.7 C 44 92 0.5 40.7 D 28 105 0.35	20.4 C	41	86
Jefferson WB-TR 0.19 14.1 B 53 72 0.87 29.1 C 324 393 0.20 15.3 B 56 76 0.91 34.1 C 339 447 0.22 18.8 B 56 98 0.83	26.1 C	317	
Signalized NB-L 0.82 53.7 D 143 204 0.88 51.6 D 165 254 0.85 53.8 D 167 236 0.91 55.1 E 174 271 0.99 80.2 F 174 271 0.87	49.8 D	171	
NB-T 0.41 28.0 C 121 185 0.28 20.6 C 76 128 0.43 26.8 C 132 199 0.29 19.5 B 82 135 0.52 64.5 E 146 226 0.47	33.8 C	109	180
NB-R 0.17 8.7 A 10 43 0.15 4.6 A 0 30 0.18 8.7 A 12 46 0.15 4.2 A 0 30 0.19 3.9 A 0 24 0.21	3.2 A	0	20
SB-L 0.16 40.5 D 13 36 0.52 47.6 D 47 96 0.16 40.5 D 13 36 0.47 42.7 D 45 93 0.09 22.9 C 10 25 0.38	27.3 C	35	69
SB-T 0.72 58.4 E 111 184 0.84 64.4 E 129 250 0.75 60.3 E 117 203 0.86 62.9 E 150 282 0.39 40.9 D 60 84 0.78	57.0 E	80	144
SB-R 0.11 3.7 A 0 14 0.74 33.7 C 138 261 0.11 3.7 A 0 14 0.69 29.8 C 133 232 0.08 0.3 A 0 0 0.74	28.9 C	106	208
Intersection - 17.0 B - - - 28.2 C - - - 18.6 B - - - 30.6 C - - - 24.5 C - - -	25.8 C		

April 2018						2030	Rase			COIN	VIDOR	I	i-10 /	111/AL I	315 K		Build								2030 Buile	d Mitigation	1			east R
			A	M Peak H	our	2030	2036	Pf	M Peak Ho	ur			A	M Peak Ho	our	2030	Duna	P	M Peak Ho	our			AM Peak Ho		2000 Dulle	viitigation		M Peak Ho	our	
Intersection	LANE GROUP	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)
	EB-LTR	0.84	36.1	D	184	317	0.45	23.1	С	62	112	0.85	36.9	D	190	321	0.46	23.3	С	63	112	0.70	20.9 C	111	243	0.45	23.1	С	61	112
	WB-LTR	0.27	13.6	В	25	58	0.34	31.1	С	155	254	0.26	14.3	В	26	57	0.80	31.5	С	160	256	0.22	8.6 A	15	48	0.79	31.1	С	155	256
E River Rd & 253	NB-LTR	0.87	26.0	С	340	532	0.48	14.3	В	119	198	0.88	26.7	С	350	563	0.49	14.4	В	123	200	0.68	Movement Does N	Not Exist	328	0.33	Movem 13.7	nent Does	Not Exist 84	139
233 Signalized	NB-LT NB-R	-	Moven	nent Does	Not Exist			Movem	nent Does N	Not Exist			Movem	ent Does N	Not Exist			Moven	nent Does N	Not Exist		0.88	3.5 A	4	48	0.33	2.7	A	0	27
	SB-LT	0.31	11.5	В	49	88	0.48	26.5	С	262	416	0.36	12.2	В	55	99	0.82	27.3	С	383	427	0.49	18.4 B	50	132	0.81	26.2	С	264	418
	SB-R	0.11	2.2	А	0	19	0.48	3.7	Α	11	53	0.11	2.2	Α	0	19	0.36	3.7	А	21	54	0.14	3.4 A	0	25	0.36	3.8	А	12	54
	Intersection WB-L	0.35	25.8 46.7	C F	+ :	36	- 1.45	22.2 325.8	C F	-	263	0.41	26.4 56.2	C E	-	- 44	1.52	22.6 358.8	C F	-	276	-	15.6 B	-	-	-	21.3	C	-	-
	WB-R	0.26	24.5	č	-	25	0.10	11.8	В	-	8	0.30	26.5	Č	-	30	0.11	11.9	В	-	9									
E River Rd &	NB-T	0.66	0.0	A	<u> </u>	0	0.28	0.0	Α	-	0	0.68	0.0	A	-	0	0.29	0.0	A	-	0									
Brooks Unsignalized	NB-R SB-L	0.10 0.04	0.0 12.0	A B	+ :-	0 4	0.05 0.08	0.0 8.9	A A	-	7	0.10 0.06	0.0 12.4	A B	-	0 5	0.05	0.0 8.9	A	-	7		No Mitigatio	on			N	lo Mitigati	on	
Unsignalizea	SB-L SB-T	0.18	0.0	A	+ -	0	0.60	0.0	A	<u> </u>	0	0.19	0.0	A	-	0	0.61	0.0	A	-	0									
	Intersection	-	2.3	Α	-	-		25.5	D	-	-		2.7	Α	-	-	-	28.0	D	-	-									
	EB-L	0.43	47.6	D	18	50	0.90	65.5	E	116	254	0.49	59.8	E	21	65	0.91	68.1	E	115	254	0.38	40.2 D	16	48	0.73	43.2	D	61	159
	EB-TR WB-L	0.06	21.9 43.0	C D	113	28 193	0.29 0.62	16.9 30.8	B C	61 114	113 199	0.06 0.82	25.8 60.2	C E	9 153	31 284	0.29 0.65	16.4 31.2	B C	59 119	110 207	0.05	19.2 B 35.9 D	7 119	26 206	0.23 0.62	14.8 28.0	B C	32 95	83 204
E River Rd &	WB-TR	0.88	44.3	D	169	333	0.62	5.0	A	8	65	0.82	70.8	E	225	430	0.48	4.9	A	7	65	0.80	31.6 C	159	314	0.50	5.2	A	6	65
Lehigh Station	NB-L	0.06	9.6	А	11	26	0.06	22.4	С	5	19	0.0.6	8.6	Α	11	25	0.06	23.2	С	5	19	80.0	12.2 B	12	30	0.06	26.1	С	4	22
Signalized	NB-TR	0.97	40.4	D	503	815	0.85	41.9	D	232	400	1.00	47.1	D	631	972	0.89	46.4	D	245	428		Movement Does N					ent Does		
	NB-T		Moven	nent Does	Not Exist			Movem	nent Does N	Not Exist			Movem	ent Does l	Not Exist			Moven	nent Does N	Not Exist		0.78	24.2 C	305	451	0.57	31.0 6.2	C	109	238 54
	NB-R SB-L	0.72	33.0	Тс	45	143	0.88	42.5	D	138	302	0.96	77.0	ΙE	82	228	0.92	51.3	Тъ	156	334	0.60	2.8 A 15.9 B	0 43	42 96	0.37	15.4	A B	87	207
	SB-T	0.69	5.4	А	32	55	0.69	19.9	В	267	395	0.14	5.0	Α	32	54	0.69	20.6	С	276	409	0.15	6.8 A	38	64	0.69	19.3	В	209	462
	SB-R	0.65	1.4	А	0	25	0.05	4.2	Α	1	16	0.23	1.2	Α	0	23	0.05	4.4	А	1	16	0.25	1.7 A	0	28	0.05	5.3	Α	1	18
	Intersection	- 0.44	33.1	С	-	- 112	- 0.10	29.3	С	-	- 40	- 0.42	46.6	D	-	- 111	- 0.10	31.8	С	-	- 40	-	19.5 B	-	-	-	18.7	В	-	-
	EB-LT EB-R	0.44	38.2 0.3	D A	59	112	0.10	21.5 0.1	C A	20 0	48 0	0.42 0.07	36.9 0.3	D A	58 0	111 0	0.10	22.1 0.1	C A	20	49 0									
	WB-LT	0.89	74.8	Е	114	244	0.73	36.5	D	142	252	0.93	79.0	E	127	266	0.75	38.5	D	149	288									
E River Rd &	WB-R	0.27	8.7	Α	0	42	0.49	5.2	Α	0	60	0.26	8.3	А	0	41	0.50	5.3	А	0	61									
Chesapeake Landing/Bailey	NB-L	0.02	9.5	A	3	11	0.13	17.7	В	8	25	0.04	10.2 37.0	В	4	14	0.14	17.5	В	8	26 346		No Mitigatio	on			N	lo Mitigati	on	
Signalized	NB-T NB-R	0.91 0.23	32.8 2.1	C A	428 0	706 30	0.67 0.32	24.2 3.4	C A	222 0	352 43	0.93 0.25	2.2	D A	451 0	732 32	0.67	23.9 3.3	C A	230	42									
	SB-L	1.04	85.6	F	166	337	0.22	8.2	A	21	45	1.05	88.5	F	167	338	0.22	8.0	A	22	43									
	SB-TR	0.38	6.0	Α	93	139	0.78	19.3	В	291	478	0.39	6.5	Α	101	150	0.78	19.0	В	303	465									
	Intersection	-	34.5	С	-	-	- 0.12	18.2	В	- 10	- 27	- 0.20	36.8	D	-	-	- 0.12	18.3	В	- 10	- 27	0.20		1 20	L 67	0.13	100			1 20
	EB-LTR WB-LT	0.29 0.13	31.6 36.5	C D	26 14	67 40	0.12 0.62	19.3 39.4	B D	10 81	37 161	0.29 0.13	31.6 36.6	C D	26 14	67 40	0.12	19.3 39.6	B D	10 82	37 161	0.29	31.4 C 36.3 D	26 14	67 40	0.12 0.61	18.8 37.6	B D	9 77	36 158
	WB-R	0.03	0.2	Α	0	0	0.17	8.8	Α	0	32	0.03	0.2	A	0	0	0.17	8.8	A	0	32	0.03	0.2 A	0	0	0.16	8.5	A	0	32
E River Rd &	NB-LTR NB-L	0.77	10.1	В	289	497	0.80	15.7	В	288	530	0.77	10.2	В	293	505	0.80	15.9		292	535	0.01	Movement Does N	Not Exist	3	0.06	5.9	nent Does A	Not Exist	11
Meadows / RIT	NB-TR		Moven	nent Does	Not Exist			Movem	nent Does N	Not Exist			Moven	ent Does N	Not Exist			Moven	nent Does N	Not Exist		0.77	10.2 B	289	506	0.78	15.1	В	269	514
Signalized	SB-LTR	0.59	6	Α	158	243	0.74	13.7	В	238	433	0.59	6.1	А	161	248	0.74	13.8	В	242	438		Movement Does N	Not Exist				ent Does	Not Exist	
	SB-L	-	Moven	nent Does	Not Exist			Movem	nent Does N	Not Exist			Movem	ent Does N	Not Exist			Moven	nent Does N	Not Exist		0.12	3.8 A	3	11	0.21	8.7	A	7	25
	SB-TR Intersection	-	9.5	ΙA	I -	-	-	16.8	В	_	-	-	9.5	I A	_	-	-	16.9	Тв	- 1	-	0.54	5.3 A 9.2 A	143	219	0.66	11.4 15.3	B B	201	371
	EB-L	0.41	11.5	В	58	66	0.19	8.8	A	4	10	0.41	11.4	В	59	67	0.20	8.7	A	4	10		J.Z A				15.5			
	EB-T EB-R	0.63 0.72	15.1 7.5	B A	200 91	313 88	0.31 0.37	5.5 1.3	A A	27 0	57 0	0.64 0.73	15.3 7.0	B A	216 91	314 87	0.33 0.38	6.0 1.4	A	28 0	58 0									
E River Rd &	WB-L	0.72	40.8	D	28	106	0.37	20.4	C	41	86	0.73	42.7	D	29	107	0.36	22.6	C	42	92									
Jefferson	WB-TR	0.22	18.8	В	56	98	0.83	26.1	С	317	385	0.22	19.3	В	57	99	0.87	29.8	С	326	447									
Signalized	NB-L	0.99	81.1	F	174	273	0.88	50.1	D	172	261	0.94	68.5	E	174	264	0.85	46.9	D	171	253		No Mitigatio	on			N	lo Mitigati	on	
	NB-T NB-R	0.53 0.19	34.5 3.8	C A	146	226 24	0.47 0.21	33.8 3.2	C A	110 0	181 20	0.52 0.19	34.0 3.8	C A	145 0	228 24	0.43 0.19	30.7 2.8	C A	105 0	173 19									
	SB-L	0.19	22.9	C	10	25	0.38	27.3	C	35	69	009	22.9	C	10	25	0.19	24.5	C	33	65									
	SB-T	0.39	40.8	D	61	85	0.78	57.2	E	80	144	0.40	41.5	D	61	86	0.66	46.5	D	78	120									
	SB-R	0.08	0.3	Α	0	0	0.74	28.9	С	106	208	0.09	0.3	Α	0	0	0.71	26.2	С	105	200									
	Intersection	-	24.7	С	-	-	-	25.9	С	-	-	-	23.1	С	-	-	-	26.0	С	-	-									

						2035	Base				
			A	M Peak Ho	ur			PI	VI Peak Ho	ur	
Intersection	LANE GROUP	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)	V/C Ratio	Delay	Level of Service	50th Queue (ft)	95th Queue (ft)
E Rîver Rd &	EB-LTR	0.72	22.7	С	122	253	0.47	24.1	С	63	115
2 53	WB-LTR	0.22	9.2	Α	16	51	0.81	33.1	С	161	283
Signalized	NB-LT	0.69	19.7	В	170	337	0.34	13.4	В	86	138
<u> </u>	NB-R	0.38	4.0	Α	10	55	0.17	2.6	Α	0	26
	SB-LT	0.49	17.9	В	53	131	0.82	26.8	С	287	438
	SB-R	0.13	3.1	Α	0	24	0.36	3.9	Α	16	56
	Intersection	-	16.3	В	-	-	-	22.2	С	-	-
	WB-L	0.46	65.7	F	-	50	1.67	431.8	F	-	295
E River Rd &	WB-R	0.32 0.72	29.0 0.0	D	-	33 0	0.10	12.0	В	-	8
	NB-T		-	A	=			0.0	A	=	0
Brooks	NB-R	0.10	0.0	A	-	0 5	0.05	0.0	A	-	7
Unsignalized	SB-L SB-T	0.06 0.20	12.8 0.0	B A	-	0	0.08	9.0 0.0	A	-	0
	Intersection	0.20	3.0	A		-	0.04	32.4	D		-
	EB-L	0.35	35.6	D	15	44	0.81	47.5	D	104	243
	EB-TR	0.35	18.0	В	7	24	0.81	15.2	В	56	112
	WB-L	0.03	36.5	D	111	24 195	0.28	27.5	С	112	209
E Dinam Del G									 	7	
E River Rd &	WB-TR	0.82	31.0	C	139	291	0.46	4.5	A C	6	63
Lehigh Station	NB-L	0.07	10.8	A	11	27	0.08	28.3			22
Signalized	NB-T	0.80	23.6	В	283	435	0.61	34.8	С	142	247
	NB-R	0.36 0.63	2.7 18.0	A A	0 37	40 112	0.38	6.3 20.3	A C	0 120	54 217
	SB-L		-								
	SB-T	0.16	6.1	В	34	59	0.78	25.7	С	304	523
	SB-R	0.25	1.6	В	0	27	0.05	6.0	A	2	19
	Intersection	-	19.3	В		- 420	- 0.40	22.3	С	- 20	-
	EB-LT	0.65	54.0 0.5	D	62	139 0	0.10	21.6	С	20 0	48 0
	EB-R	0.08		A	0		0.03	0.1	A		
5 pt p.l.6	WB-LT	1.17	155.6	F	154	296	0.74	37.6	D	151	279
E River Rd &	WB-R	0.30	9.5	A	0	43	0.49	5.2	A	0	60
Chesapeake Landing/Bailey	NB-L	0.04	10.1	В	4	14	0.15	18.6	В	9	27
Signalized	NB-T	0.99	48.4	D	530	827	0.68	24.7	С	235	363
	NB-R	0.25	2.1	Α	0	32	0.32	3.4	A	0	43
	SB-L	0.92	54.2	D	143	306	0.22	8.4	А	22	45
	SB-TR	0.38	5.2	Α	87	130	0.80	20.5	С	318	504
	Intersection	-	43.6	D	-	-	-	18.9	В	-	-
	EB-LTR	0.16	31.6	C	27	67	0.12	19.2	В	9	37
r pt nJ o	WB-LT	0.16	36.7	D	15	41	0.63	39.1	D	80	163
E River Rd &	WB-R	0.16	0.2	A	0	0	0.17	8.7	A	0	32
Meadows / RIT	NB-L	0.82	2.4	A	1	3	0.06	5.7	A	3	11
Signalized	NB-TR	0.82	10.6	В	300	533	0.79	15.1	В	280	511
	SB-L	0.82	4	Α	3 147	11	0.21	8.6	A	7	24
	SB-TR Intersection	0.82	5.3 9.4	A	147	225	0.67	11.3 15.4	B B	209	368
	EB-L	0.44	5.9	A	44	- 46	0.19	8.3	A	4	11
	EB-T	0.63	8.1	A	204	211	0.32	5.8	A	28	57
	EB-R	0.71	3.6	Α	57	62	0.38	1.4	Α	0	0
E River Rd &	WB-L	0.49	30.5	С	24	71	0.36	22.9	С	41	95
Jefferson	WB-TR	0.19	12.4	В	46	64	0.87	30.0	С	318	465
Signalized	NB-L	0.83	47.0	D	150	214	0.92	57.0	E	177	277
	NB-T	0.58	35.8	D	130	259	0.44	31.3	С	106	175
	NB-R	0.20	3.3	Α	0	20	0.20	2.9	Α	0	19
	SB-L	0.11	23.8	С	9	26	0.33	24.2	С	33	66
	SB-T	0.65	51.1	D	57	92	0.61	43.5	D	77	118
	SB-R	0.11	0.5	Α	0	0	0.68	23.9	С	104	188
	Intersection	-	16.7	В	-	-	-	27.1	С	-	-

FIGURE 5.5 – 2035 LOS Table

Intersection Capacity Improvements

<u>2015</u>

The NB approach to Jefferson Road is currently experiencing capacity issues with queue's extending beyond the left turn lane storage. This is the only intersection with capacity issues under 2015 conditions.

2020

A substantial amount of development is included in the 2020 volumes with the largest impacts to the Lehigh Station and Bailey Road intersections. Traffic heading toward either I-390 or West Henrietta Road is reflected in the need for NB right turn lanes.

The increased queue lengths at the Bailey Road/Chesapeake Landing Intersection will extend beyond the existing storage bays requiring increases to the current storage lengths for the NB left turn, SB left turn, WB.

As previously shown in Table 7 the Lehigh Station intersection includes the re-alignment of the former Kodak Riverwood Campus northern driveway which provides a new EB approach. The new EB approach would include a left turn lane and a thru/right lane. It is anticipated that the developer would construct the new EB approach and turn lanes at this intersection due to the proposed driveway re-alignment.

A SB left turn lane, NB right turn lane and WB right turn lane are proposed at the unsignalized Brooks Road intersection. Unsignalized analysis predicts tight conditions but often over estimate delays. The need for a signal should be monitored as traffic conditions change.

The SB right turn lane at the Erie Station intersection should be extended due to the potential for SB thru queues to block access to the right turn lane.

2025

As the volumes increase to 2025 we incur additional delays on the WB approach at the unsignalized Brooks Road intersection. Increasing the WB right turn lane length will improve LOS for this approach however the left turn movement is LOS 'F' and the need for signalization should be monitored as projected development progresses.

A new 500' NB right turn lane at the Bailey Road/Chesapeake Landing intersection will be required due to increases in volume.

Traffic at the north end of the corridor has increased requiring a 2nd SB thru lane at Jefferson Road to provide sufficient capacity at this congested intersection.

2030

Under 2030 conditions the Erie Station intersection requires a new NB Right Turn lane.

As stated under 2025 conditions the need for signalization at Brooks should be monitored as the WB approach at Brooks is LOS 'F'.

With additional student housing and increased thru volumes the RIT driveway now includes NB/SB left turn lanes and an increased WB right turn lane.

Intersection Capacity Improvements (cont.)

<u>2035</u>

The Town has not identified any long term development post 2030 therefore there are no potential intersection improvements in 2035.

Roundabouts

An alternative planning solution that may also address the proposed intersection capacity mitigation is installation of a roundabout. Roundabouts typically occupy a larger footprint than a standard 4-way intersection thus requiring additional Right-of-Way for construction but this may be possible where developer owned lands are located adjacent to the proposed intersection. Overall costs may be similar due to avoiding the construction of turn lanes and signalization. Due to topography some of the intersections may not be practical for a roundabout as may be the case at the Lehigh Station Road intersection.

Conceptual Improvements

Concept sketches were developed for each of the identified intersection capacity improvements based on the following design parameters:

- Design Speed = 45 mph
- Lane Width = 11'
- Length of Shift Tapers = Width * Speed (45mph)
- Turn Bay Tapers = 75'
- Minimum shoulder width = 6' (8' desirable per NYSDOT HDM Exhibit 2-3)

Intersection improvement concepts for each analysis year are shown in Appendix C. These concepts do not include any required highway improvements at the future development driveways (except at Lehigh Station Road) which may require additional roadway improvements/turn lanes as required by MCDOT.

Linear Corridor Analysis

HCM Exhibit 10-7 provides example service volumes for Urban Streets based on Roadway Class I thru IV. Roadway class is defined by free flow speed/signal spacing/cycle length etc. Based on the signal spacing and free-flow speed characteristics of East River Road, it would fall somewhere between a Class I and Class II roadway. For a 2-lane roadway LOS 'D' is estimated as 2,030 veh/hour for Class I and 1,700 veh/hour for Class II roadways. The 2-way year 2025 full build peak hour volumes are more than 1,500 vehicles north of Brooks Road with a maximum of ~1,800 vehicles at the River Meadow Drive/Farnum Lane(RIT) intersection. Based on the volumes in HCM Exhibit 10-7 the future conditions are approaching the capacity of a 2-lane roadway.

The areas with significant driveways that may benefit from a TWLTL include the segment between the Farnum Lane/River Meadow intersection and the southern unsignalized RIT driveway. In this section, there are more than 21 driveways per mile, several side streets and both a horizontal and vertical curve that may limit sight distance.

Intersection improvements were identified based on maintaining acceptable LOS conditions and where volume thresholds were exceeded for turn lanes or signalization. See Table 7 below for a summary of intersection capacity improvements.

Table 7 – Summary of Intersection and Corridor Capacity Improvements						
Intersection	Recommended Capacity Improvements					
East River Road - CR 84 at:	2015 (Existing)	2020 (Full Build)	2025 (Full Build)	2030 (Full Build)	2035 (Full Build)	
Erie Station Road - NYS 253 (NYSDOT Intersection)	None	> Extend SB RT Lane by 225' from 200' to 425'	No Additional	➤ New 300' NB RT Lane	No additional	
Brooks Road - CR 77 (MCDOT Intersection)	None	 New 100' SB LT Lane New 100' NB RT Lane New 150' WB LT Lane 	The unsignalized capacity analysis predicts potentially long delays for the WB left turn movement. However unsignalized analyses are known to exaggerate such delays therefore it is recommended that this unsignalized intersection is monitored for a possible traffic signal as traffic conditions change.			
Lehigh Station Road - CR 79 (MCDOT Intersection)	None	 New 450' NB LT Lane. Re-stripe SB LT Lane Re-stripe SB RT Lane 200' WB LT Lane. 200' EB LT Lane (New driveway). Remove Riverwood Drive (Development). Replace Traffic Signal	No additional	> New 450' NB RT Lane	No additional	
Bailey Road - CR 81/Chesapeake Landing (MCDOT Intersection)	None	 Extend NB Left Turn Lane via re-striping T-W-L-T-L Extend SB LT Lane by 355' from 120' to 475' Extend WB RT Lane by 110' from 190' to 300' 	➤ New 500' NB Right Turn Lane.	No additional	No additional	
River Meadow/Farnum Lane (RIT) (MCDOT Intersection)	None	None	None	 New 500' NB LT Lane New 350' SB LT Lane Extend WB RT by 100' from 70' to 170' 	No additional	
Jefferson Road - NYS 252 (NYSDOT Intersection)	None	➤ New 2nd NB LT or Thru Lane	➤ New 2 nd SB Thru Lane.	No additional	No additional	
Corridor Improvements	Recommended Corridor Improvements					
Farnum Lane to Minett Drive	None	None	Two-Way-Left-Turn-Lane	None	None	
Sidewalks	Future Sidewalks are a long term improvement goal due to environmental and R.O.W. constraints					

Improvement Funding Legend

Developer Cost Sharing Funded Improvements
Kodak Riverwood Development Improvements
MCDOT Improvements

CONCLUSIONS AND RECOMMENDATIONS

Corridor Capacity and Safety Improvements

The intersection improvements noted above will need to be incorporated prior to the respective development year in order to maintain acceptable LOS conditions along East River Road. Where incremental increases in turn bay lengths are required the full storage lengths should be constructed to minimize construction impacts. Each individual development may also require a separate traffic study to identify any additional capacity or safety improvements necessary at each development driveway, as determined by MCDOT.

A two-way left turn lane (TWLTL) is recommended in areas with thru volumes exceeding 900 vehicles/hour that also contain a concentration of residential and commercial driveways. Without a TWLTL left turning vehicles in these areas will block thru traffic which may increase the likelihood of rear-end crashes. In looking at the entire corridor the segment with the highest concentration of residential and commercial driveways is located between the Farnum Lane (RIT)/River Meadow Intersection and the unsignalized southern RIT driveway. Horizontal and vertical curves also limit visibility of left turning vehicles. A 3-lane roadway section would begin at the southern end as an extension of the proposed SB left turn lane at Farnum Lane and end with a new SB left turn lane at the unsignalized RIT driveway. This 3-lane section would be in addition to the intersection improvements identified above. Widened shoulders are already being installed between the Bailey Ave/Chesapeake signal and Farnum Lane/River Meadow intersection. A concept for a 3-lane section (TWLTL) is shown in Appendix C. Additional linear capacity in other areas south of Farnum Lane/River Meadow may be necessary dependent on future volumes and driveway locations.

Pedestrian Accommodations

Pedestrians are currently using the existing shoulders to travel within the East River corridor due to a lack of sidewalks. Pedestrian volumes are expected to increase with future residential development and proximity to RIT. As noted in the existing conditions sections, there are several current residential developments that provide sidewalk connections to East River Road. In such a situation, ADAAG requires that accessible design standards be used to design the segments of the shoulders that are intended to serve as walkways. This means that these segments of the shoulder must meet the ADAAG 2% maximum cross slope requirements. This conflicts with the normal 6% shoulder cross slope and may result in technical infeasibilities where the cross slope of the traveled way exceeds 2% (i.e., the insides of curves). The Town of Henrietta Active Transportation Plan identified the following priority sidewalk areas within the project corridor:

- East Side of East River Road from Brooks to Farnum Lane
- West side of East River Road from Farrell Road Extension to River Meadow Drive

Installation of new sidewalks should be a priority in this corridor given the future residential, student housing and senior housing development however this may be more of a long term goal due to funding concerns and the potential environmental/R.O.W. impacts. Both the Town and County consider improved shoulders a short term solution that may be beneficial in areas with a two-way left turn lane where vehicles are not encouraged to by-pass left turning vehicles on the shoulders. Future sidewalk costs should include installation of x-walks and pedestrian signals at the existing signalized intersections. Future pedestrian signal timings were not included in the capacity analysis.

Bicycle Accommodations

The existing shoulders are currently being used by bicyclists within the corridor with the number of bicyclists expected to increase with development. Per the NYSDOT Highway Design Manual (HDM) shoulders that are well designed for bicycling will also have maintenance, safety and other benefits that affect other highway users. AASHTO defines paved shoulder, together with the adjacent travel lane, as shared use facilities on roadways where bicycling is permitted. The minimum shoulder width on a project to specifically accommodate bicycling should be 4 ft. The HDM states that roadways that include long, steep grades should also consider providing additional width on the downhill shoulders. Bicyclists traveling downhill frequently will reach high rates of speed and may find that narrow shoulders 4 ft. or less, are unusable when debris and litter have accumulated on them, or bicyclists may not trust unseen shoulder conditions ahead. Consistent shoulder widths in excess of 4' should be considered within the corridor due to rolling terrain and the expected increase in users. The planned shoulder width increases between Fairwood Drive and Farnum Lane will improve bicycle conditions in this area. Widening the shoulders north of Farnum Lane should also be considered.

Transit Service

Providing transit service along East River Road should be evaluated as student housing options grow in the corridor and in concert with any future corridor pedestrian accommodations.

Other Considerations

The East River Road corridor includes other existing driveways and geometric features that should be reviewed as volumes and development increase. Several existing unsignalized intersections within the corridor may experience significant delays due to future corridor volumes. Some of these impacts cannot be avoided however access restrictions could be implemented where alternate signalized access exists. RIT for example has three (3) driveways along East River Road with the two (2) northern most driveways being unsignalized. Southbound vehicles exiting RIT have the option of using the traffic signal at Farnum Lane. This alternate access would facilitate either restricting left turns onto East River Road from the two (2) northernmost driveways or providing signing directing Southbound vehicles to the Farnum Lane exit. This could also apply to the Still Pond/Cape Henry/Chesapeake Landing Residential neighborhood.

Sight distance along the corridor is limited in areas due to the rolling terrain. Any future roadway improvements and future development driveways will need to take this into consideration and any remaining non-standard features justified thru the approval process.

COST SHARING

Highway improvement cost sharing is a mechanism that allows for both public and private investment to maintain a safe and efficient highway over the next 20 years. As development occurs along East River Road one way to mitigate for the projected capacity improvements is to develop equitable cost shares based on trip generation rates and development area. Working with the Town and County this process will define 'fair' shares that include both existing capacity improvements (Town/County responsibility) as well as development generated capacity improvements. A Transportation Development District (TDD) is the anticipated funding mechanism for these improvements. Estimated costs for each improvement are shown below in Table 8.

Table 8 – Opinion of Probable Costs for Corridor Improvements					
Intersection	Cost				
East River Road (CR 84) at:					
Erie Station Road (NYS 253)	\$420,000				
Brooks Road (CR 77)	\$640,000				
Lehigh Station Road (CR 79)	\$150,000				
Bailey Road (CR 81)/Chesapeake Landing	\$650,000				
River Meadow/Farnum Lane (RIT)	\$770,000				
Jefferson Road (NYS 252)	\$2,700,000				
TWLTL: Farnum Lane/River Meadow to Minett Drive (RIT)	County Funded				
Total =	\$5,330,000				

Note: Does not include R.O.W., Private Utility relocation, Wetlands/Environmental Mitigation or NYSDOT Permit Fees. 2016 Costs.